

**California Department of Transportation
Structural and Seismic Analysis Section**

**PreProcessor for
Static and Dynamic Analysis of Concrete Bridges**

B A G

Instructions for Users

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CHAPTER 11, BAG---TABLE OF CONTENTS

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INTRODUCTION

GENERAL DESCRIPTION

BAG (Bridge Analysis Generator) is a preprocessor program which generates the McAuto STRUDL input coding (1.1) required to perform a response spectrum dynamic analysis and a static analysis on concrete bridge structures. BAG generates the structural model, loading functions and output requests for a dynamic and static analysis using a minimum amount of geometric and structural input data. The assumptions inherent in the generated model are based on extensive bridge design and analysis experience and are further verified by detailed case studies (1.2,1.3). The STRUDL coding generated by BAG does not run in an ICES-STRUDL environment. The generated STRUDL input coding is easily verified, modified, and saved prior to STRUDL processing. The Caltrans (1.4) (CALifornia Department of TRANSportation) support motions, or design response spectra are stored as STRUDL files and are referenced directly from BAG. Special response spectra may also be input.

The seismic analysis and design of structures is still a relatively new field, employing many engineering disciplines which have their own associated assumptions and limitations. It is important that the user recognize this in formulating a particular analysis. By verifying the applicability of the generated models, the user can insure an accurate and economical seismic analysis.

Lateral loadings, dead loads (for single column bents only), and distribution factors for use in the YIELD program are generated for all of the bents in the structural model. Lateral loads include wind, wind on live load, longitudinal force, temperature, and centrifugal force. Default values are provided in BAG for wind, wind on live load, longitudinal force, and temperature. If the default values are used, the forces and moments generated from the BAG model must be modified using a ratio of the actual forces applied to the structure divided by the applied forces. The user may supply the actual forces and moments appropriate for the structure by modifying the STRUDL coding.

STRUCTURAL MODELING

Idealizing a structure in the form of a mathematical model for a static or dynamic analysis requires the input of the correct structural stiffness parameters. In addition, the correct mass distribution to model the vibrational characteristics of the overall system must be included in the dynamic analysis. The BAG model provides the required number of joints, using default input values, to produce a model consistent with the state-of-the-art for a linear dynamic analysis of a typical bridge structure. The more experienced user may elect to specify the number of required joints to provide a better or more practical model for a particular bridge.

The generated structural model is a space frame composed of straight beam elements with six degrees of freedom at each joint. The structural mass is lumped at each of the generated joints to model inertia effects. Only the three translational inertia effects are considered (the rotational inertia effects are combined with the translational inertia effects in order to reduce the computer memory requirement) in the dynamic analysis. BAG generates the intermediate span and column joints required to model these inertia effects. Additional joints at one foot centers are inserted at abutments and intermediate expansion hinges in order to prevent loss of a portion of the mass at the released end of the member. The program provides default values of 3 joints per superstructure span and 2 joints per column. The lumped mass terms are computed from the member tributary areas using a weight density of 150 lbs mass/cu. ft.

SUPERSTRUCTURE

The horizontal alignment of the bridge can be tangent, curved, or a combination of both, with a maximum of two curves. The bridge alignment is considered to correspond to the centroidal axis of the superstructure. Superstructure elevations are specified at the end abutments and intermediate bents. Elevations of generated intermediate superstructure joints are computed by straight line interpolation from the specified elevations. Curved portions of the superstructure are modelled as a series of straight line elements. The generated members within a single span are all prismatic. The user may, however, specify different section properties and modulus of elasticity for each span. A maximum of 48 spans is permitted for any one BAG run.

Forked or 'Y' superstructures or structures with more than two curves may be analyzed by generating two or more separate models and merging the resulting STRUDL coding.

The procedure for multiple models is as follows:

A normal BAG run is executed to describe the mainline of the structure. This run results in joints and members being described starting at Abutment 1. The actual Abutment coordinates may be input or the default value used. An Abutment to Abutment bearing may also be input for this model (or the default used).

A second BAG run is performed for the branch portion of the bridge. This model is generated with different joint and member numbers by specifying a beginning abutment number that is higher than any span number in the previous model. The coordinates for this abutment are also specified along with the bearing of the abutment to abutment chord.

After both STRUDL data files are generated, the files are combined. Duplicate commands are then stripped from the combined file leaving a complete file ready for execution.

More than one additional model may be added using this procedure. A separate BAG run is required for each additional model.

ABUTMENTS

Bridge abutments may be modeled to account for bearings, soil- foundation interaction and connectivity to the superstructure. Of the six possible degrees of freedom or restraint conditions at the abutment, two are set in the generated model and four are included in the abutment input data. The abutment releases beyond the users control are the rotations about the vertical axis and the centerline of bearing at the abutments. These two rotational degrees of freedom are included in the STRUDL model as moment releases. The remaining four degrees of freedom, three translational and the one rotational about the centroidal axis of the superstructure(i.e., torsion), may be specified in the input data. These four degrees of freedom may be specified as either fixed or free and, in addition, the two horizontal translational degrees of freedom may be represented by linear springs to help model the soil-structure interaction effects. The user may elect, for these four degrees of freedom, to use the default force releases which are described under ABUTMENT RELEASES on the GENERAL DATA PANEL - 2.

BENTS

The bents may be composed of single or multiple columns up to a maximum of 9 columns per bent. The individual columns in a multiple-column bent are connected to horizontal cap members framed into the superstructure at the specified ele-

vation for the bent. The bents may be skewed with respect to the superstructure, or normal(or radial) to the defined superstructure alignment. Foundation effects may be inputted by specifying spring coefficients at the column bases. The spring coefficients can also be added in later to the STRUDL commands by the engineer. Individual columns may be modeled as a series of prismatic sections to simulate a variable cross section. Up to 10 column types may be used for any structure, although only one column type is permitted for each bent.

INTERMEDIATE EXPANSION HINGES

One intermediate expansion hinge may be included in each span of the superstructure. The hinge is always considered normal to the superstructure and releases are made as member releases relative to the superstructure. Releases at a hinge are always modeled the same i.e. FORCE X (longitudinal), MOMENT Z (normal bending), and MOMENT Y (transverse bending) released. ALL other forces and moments i.e. FORCE Y (vertical), FORCE Z (transverse), and MOMENT X (torsional) are considered fixed. Two joints spaced at 1.0 foot are used to model the hinge. All releases are made at the end of the 1.0 foot long member. The hinge joint location described in the input is always located toward the closest bent, with the second joint located 1.0 foot away toward the farthest bent. Seismic restrainer cables may be placed in the bridge superstructure to prevent excessive joint openings during an earthquake. The inclusion in of restrainers in a linear elastic analysis is, however, an approximation because they are assumed to work in compression as well as in tension. Restrainers are modeled as a single truss element which connects the closest adjacent joints. The area of the restrainer used in the analysis is adjusted to provide the same stiffness as the input data requested i.e. L/A of the model equals the input L/A . Comparisons with nonlinear analyses indicate that this will give a fairly good approximation of the overall structural response usually on the conservative side. It is required that restrainer forces be designed by the simplified procedure (see Memo 15-10 and Bridge Design Specifications 3.21.6.1).

PHASE-II RETROFIT

BAG will generate additional nodes, members, and commands for the Phase-II retrofit structural model. The STRUDL commands preceded by a '\$' sign give the users one method for making the necessary changes in the structural model required in the Phase-II retrofit analysis.

LOADING

CALTRANS earthquake response spectra are used by the program. CALTRANS earthquake response spectra are unreduced elastic curves with 5% damping. The force levels obtained from the CALTRANS curves are elastic forces which have NOT been reduced for risk and ductility. Earthquake ground accelerations are normally assumed to be directed parallel and perpendicular to a straight line between the abutment joints. Up to seven other directions in the horizontal plane may be optionally specified if desired.

Static loads can be supplied by the engineer or the engineer can use the default values. If the default values are used the engineer must prorate the moments and shears according to the actual loads.

REFERENCES

- 1.1 "ICES STRUDL", by McDonnell Douglas Automation Company, M6340040, April, 1983.
- 1.2 Imbsen, R. A., Nutt, R. V., and Gates, J. H., "Seismic Design of Highway Bridges-Workshop Manual", Federal Highway Administration, U. S. Department of Transportation, Contract No. DOT-FH-11-9426, February, 1979.
- 1.3 Imbsen, R. A., Nutt, R. V., and Penzien, J., "Seismic Response of Bridges-Case Studies", Report No. UCB/EERC-78/4, Earthquake Engineering Research Center, University of California, Berkeley, June, 1978.
- 1.4 Standard Specifications For Highway Bridges - Thirteenth Edition 1983, AASHTO with revisions by CALTRANS, DIVISION of STRUCTURES, Jan, 1985, pgs 29A-29D.

INPUT PANEL INSTRUCTIONS

PANEL DATA EDIT

There are a total of 9 different panels in this program. Only 10 PF keys are defined for the panel operation and 5 out of these 10 keys are defined for direct access to 5 of the 9 panels. Other panels in SPAN and BENT data can only be access by PF7(backward) or PF8(forward) keys.

- PF1 - Run data & Earthquake data panel
- PF2 - Geometry & Alignment data panel
- PF3 - Quit panel without save
- PF4 - Span data panel
- PF5 - Bent data panel
- PF6 - Partial save without check
- PF7 - Page backward
- PF8 - Page foreward
- PF9 - File
- PF10 - Column data panel

In addition to the above PF keys, there are other keys defined for easy operation. The following keys do not apply when using general data panel 1 & 2. These keys only work if it is input at the beginning of each line.

- A - Add a line
- I - Insert a line
- R " - Duplicate current line
- D - Delete current line

All bearing angles should be entered as 6 digit integers.

RUN DATA:

```
*****  
*****                               BAG                                V 4.00 RELEASE SEPT 1, 1989  
*****  
***** RUN DATA  
  
***** COMMENTS ==> _____  
***** NOM NJS NJC JMAX NSPX      SF     PLOT ANALYSIS ROTATION REDUCE I/O CONDEN.  
***** -   -   -   -   -       -         -         -         -         -  
*****  
***** NOM : NO. OF MODES      ( DEFAULT = 12 OR 3 * NO OF SPANS )  
***** NJS : NO. OF EXTRA JOINTS ADDED PER SPAN ( DEFAULT = 3 )  
***** NJC : NO. OF EXTRA JOINTS ADDED PER COL. ( DEFAULT = 2 )  
***** JMAX: MAX. NO. OF JOINTS                      ( DEFAULT = 75 )  
***** NSPX: MAX. NO. OF SPANS                        ( DEFAULT = 48 )  
***** PLOT: PLOT LENGTH CODE ( LENGTH = 18 + 6 * PLOT INCHES, 0 FOR NO PLOT)  
***** ANALYSIS: 0 = STATIC & DYNAMIC , 1 = DYNAMIC ONLY , 2 = STATIC ONLY  
***** ROTATION: 0 = YES                          , 1 = NO  
***** -----  
***** EARTHQUAKE LOAD DATA  
***** PEAK : CURVE:          UP TO 6 ADDITIONAL OPTIONAL DIRECTIONS FOR GIVEN LOAD  
***** ROCK : DES.:           :             :             :             :  
***** ACC. :   :           2           :           3           :           4           :           5           :           6           :           7  
***** 1/10 G: A-D :           :             :             :             :             :             :  
***** -----  
***** SPECIAL SITE SPECTRUM ----- (8 CHARACTERS NAME FOR STRUDL) -----  
***** F1/RUN F2/GEOM F3/QUIT F4/SPAN F5/BENT F6/PSAVE F7/PB F8/PF F9/FILE F10/COL  
*****  
*****
```

Enter a maximum of 24 alphanumeric characters to identify your problem. The comments are utilized as the ('title') for the STRUDL statement and for all dynamic loads.

Enter the number of modes to be included in the response analysis computation. If this field is left blank, the program defaults to 3 times the number of spans or 12, whichever is greater.

Enter the number of extra joints to be added to each span in order to model the inertia effects.

The program automatically inserts 0 to 6 equally spaced extra joints per span (or long hinge). The user specifies the number of extra joints, using one of the three options described in

Table 1. A zero or blank in this field automatically inserts 3 additional joints per span.

| NJS (input) | No. of Joints added per Span |
|-------------|------------------------------|
| 0 or blank | 3 (default) |
| 1 - 6 | 1 - 6 |
| 7 - 9 | 0 |

Table 1

NJC (NUMBER OF EXTRA JOINTS PER COLUMN)

Enter the number of extra joints to be added to each column in order to model the inertia effects.

The program automatically inserts 0 to 4 equally spaced extra joints per column. The user specifies the number of extra joints, using one of the three options described in Table 2. A zero or blank in this field automatically inserts 2 additional joints per column.

| NJC (input) | No. of Joints added per Column |
|-------------|--------------------------------|
| 0 or blank | 2 (default) |
| 1 - 4 | 1 - 4 |
| 5 - 9 | 0 |

Table 2

JMAX (MAXIMUM NUMBER OF JOINTS)

Enter the maximum number of joints describing the structure. The default value used is 75 joints.

This value is used as a safety protection to suppress the execution of the dynamic analysis of large problems. The user may elect to specify any number of joints up to the maximum of 1853 (i.e., 1600 superstructure and 253 substructure joints) permitted by BAG. If the specified or default value is exceeded, BAG will insert a SCAN ON command which will inhibit the dynamic analysis in the subsequent STRUDL run. In addition, BAG will insert STATISTICS ANALYSIS commands to provide the user with an estimate of the computer resources required for the STRUDL dynamic analysis. The user may purposely elect to inhibit the analysis by specifying only 1 joint for JMAX. Experience has shown that when the number of joints exceeds 75, computer resources required for the analysis need some adjustment for a special run. It is always a good practice to inhibit a complete run by specifying a '1' for large problem.

By specifying JMAX less than the number of joints generated, only the dynamic analysis portion of the STRUDL run is suppressed; all generated geometry, releases, plots, etc., will be computed and output. Execution of the run can be accomplished by removing the SCAN ON line from the STRUDL coding. Execution of the special run should be made after consultation with the Structural and Seismic Analysis Section.

NSPX (MAXIMUM NUMBER OF SPANS)

Enter the number of spans describing the structure. The default value is 48 spans.

The default value of 48 spans is specified in the program and cannot be overridden. However the user should give some thought to the special modeling considerations required for a bridge containing 10 or more spans. If there are more than 48 spans in a run, the user should consult with the Structural and Seismic Analysis section (SASA) to obtain a special run.

SF (SPECTRAL MODIFICATION FACTOR)

All values of the elastic response spectra will be multiplied by this factor. If this is left blank, a default value of 1.0 will be assumed.

PLOT (STRUCTURE PLOT)

A '0' or blank indicates that no structure plot is desired. If a plot is desired, the user must enter a number of 1 or more in this field. A three-dimensional plot of the generated structure geometry will be plotted.

The maximum horizontal length of the plot will be 18" plus 6 times the plot input number (for example, if PLOT = 1 the maximum length will be $18" + 6(1) = 24"$). The maximum vertical dimension is always 8 inches. Normally a 1(24") will provide a satisfactory plot. Very long and low bridges would require a longer horizontal dimension to adequately show the model. For example, a 1,000 foot long bridge, 20 feet high when scaled to 24" length would only be 0.48" high. By specifying PLOT = 9, the maximum length would be $18 + 6(9)$ or 72" with a height of 1.44".

BAG generated plot commands will be inserted if any value other than a '0' or blank is entered in the plot field, however, no plot will be generated unless a plot request is specified when executing the STRUDL program.

ANALYSIS

An entry in this field enables the user to determine the type of analysis desired. The default value of '0' or blank creates all of the STRUDL commands to run both a static and dynamic analysis. A '1' in this field generates the STRUDL commands to

run only a dynamic analysis. A '2' in this field generates STRUDL commands to run only a static analysis.

ROTATION

A '0' or blank in this field permits the structure to be rotated in a horizontal plane so that the centerline of the structure or the chord between the abutments coincides with the GLOBAL X axis. A '1' in this field prohibits the rotation of the structure if NO ROTATION ANGLE is given on GENERAL DATA PANEL - 2 and should be used when creating a model composed of several BAG runs. In general the initial BAG run will be rotated. The default value is '0'.

REDUCE I/O

Not used at present.

CONDENSE

Not used at present.

EARTHQUAKE LOAD DATA

PEAK ROCK ACCELERATION

Specify the peak rock acceleration, given in tenths of the acceleration of gravity, ranging from 1 to 7 (i.e., 0.1 to 0.7).

CURVE DESIGNATION

Indicate the response spectrum curve to be used representing the depth to rock like material, as shown in Table 3. Only curve designations A through D will be accepted.

| Depth to "Rock-Like Material | Curve Designation CALTRANS |
|------------------------------|----------------------------|
| 0- 10 ft. | A |
| 11- 80 ft. | B |
| 81-150 ft. | C |
| More than 150 ft. | D |

Table 3 - Response Spectrum Curve Designation

DIRECTIONS RESPONSE SPECTRA LOADINGS

The first loading for Direction 1 will be oriented both parallel and perpendicular to the global X and Z axes. Optionally up to six additional load directions may be specified by entering their direction as a bearing. Each additional load generation is a separate unique STRUDL run. This is necessary because each new direction requires rotation of the structure in order to orient the global X axis parallel to the specified direction.

Only one plot and/or reduced SCAN ON run will be generated for multiple load runs.

Each separate STRUDL problem is generated with a FINISH at its end. Only one STRUDL JOB may be executed at a time with a condense, so the user must bypass and execute each loading separately. The load name is the azimuth direction relative to the original geometry of the bridge. If structure is forced to rotate by the input angle or is not allowed to rotate, enter addition response spectra loading, then result is unpredictable.

SPECIAL SITE SPECTRUM

Specify the special site response spectrum name if it is available from the STRUDL system. If this name is inputted, the BAG will use this name in the coding generation instead of the peak rock acceleration and curve designation. Current special site spectrum are '140-195', '195-280', '280-640', and '640-720' of the century freeway.

BEARING

If the bridge is straight only the tangent bearing (Deg,Min,Sec) is required. If the structure is curved enter the bearing of the BC to PI and the remainder of the curve data required.

ROTATION ANGLE

If a '0' or blank is in the ROTATION field on the previous panel the user may specify the angle the structure is to be rotated through.

If no value is specified, BAG will rotate the structure based on the description on ROTATION.' Otherwise, BAG will rotate the structure based on the entered angle (clockwise as 'positive').

CURVE DATA

A maximum of two curves can be input. Zero radius curves are not permitted. Tangent bearings for single curves resulting in an internal angle (i.e., delta) equal to or greater than 180 degrees is not permitted. Negative stationing is not permitted.

CURVE NUMBER

The curve number is specified on the panel. If only one curve is required omit the input for the second curve.

DIR(DIRECTION OF CURVE)

Specify whether the curve is deflected to the left (L) or right (R) as viewed looking ahead on stationing.

RADIUS

Specify the radius of the centerline of the bridge in feet.

BC(BEGINNING OF CURVE) STATION

Specify the beginning of curve (BC) station in feet.

BEARING

Specify the tangent bearing (Deg,Min,Sec) from the point of intersection (PI) to the end of curve.

ABUTMENT DATA

The abutment data is used to specify the abutment number, location, geometry and support condition at the abutment.

ABUTMENT

The beginning abutment number may be supplied by the engineer or the default value used. The default value is 1. In general the default should be used unless forked or 'Y' bridge structures or structures with more than two curves are to be analyzed.

ABUTMENT STATION

The Abutment station in feet at the centerline of the beginning Abutment is given to the nearest tenth-foot. Stationing must increase from the beginning abutment to the end abutment.

ABUTMENT ELEVATION

Specify the actual elevation at the center of gravity of the superstructure in feet or assume an elevation to establish the datum. Only one reference datum is allowed. Abutments and bents must be referenced to the same datum. Zero elevation is not permitted.

ABUTMENT BEARING

The user may specify the bearing (Deg,Min,Sec) of the abutment centerline or leave this field blank. When this field is left blank the abutment centerline bearing will be assumed to be normal to the superstructure centerline (radial on a curved alignment). The bearings at the abutments and bent centerline may be specified in either direction. Bearing specifications resulting in 90 degree skews are not permitted.

Values greater than 90 degrees, or 59 min., or 59 sec. are not accepted.

ABUTMENT RELEASES

Two of the six support constraints at the abutment are assumed to be released as shown in Figure 1a. The user has control over three of the four remaining support constraints (i.e., transverse and longitudinal forces, and torsional moment about the superstructure center of gravity). The user may specify values for these forces and moments or the user may elect to use the default values by leaving the field entirely blank. Currently, two constraints (i.e., transverse and longitudinal forces) may also be described using spring coefficients in kips/in. as indicated on the input panel and shown in Figure 1b. Fixed conditions are indicated by '1' and released conditions by '0'. Any other input configuration of releases will use all of the input values (i.e., no defaults) except the vertical force will never be released. Shown in Table 4 are the abutment constraint specifications that indicate those conditions that are assumed within the program (i.e., no user control), those assumed by default (i.e., entire field blank, and those which may include

elastic spring coefficients. Abutment force releases or forces for the fixed condition are as shown in Figure 1c.

| Constraint | Assumed default | Spring Coefficients |
|-------------------------------------|-----------------|---------------------|
| Longitudinal force | Free (0) | Enter value |
| Transverse Force | Fixed (1) | Enter value |
| * Vertical Force | Fixed (1) | --- |
| Torsional Moment | Fixed (1) | --- |
| * Transverse Moment | Free (0) | --- |
| * Vertical Moment | Free (0) | --- |
| * No User CONTROL of These Releases | | |

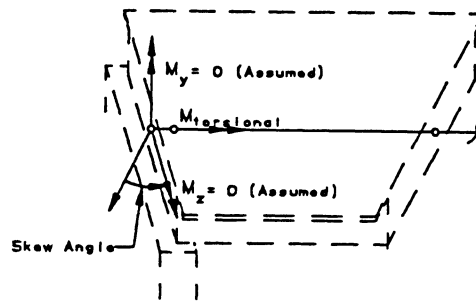
Table 4 - Abutment Constraint Specifications

END ABUTMENT

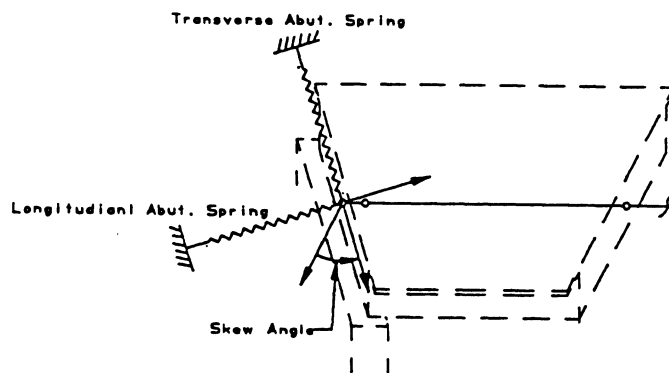
This portion is used to describe the end abutment. The same data required for beginning Abutment is also required for the End Abutment except end abutment number.

DESIGN TEMPERATURE RANGE

The design temperature range used here is for concrete structures only. For steel structure, user should modify the range by the difference between the two materials (see BDS Article 3.16). The default value is 35 degrees.



a) Assumed Release Conditions



b) Permissible Spring Coefficients & Coordinate Directions

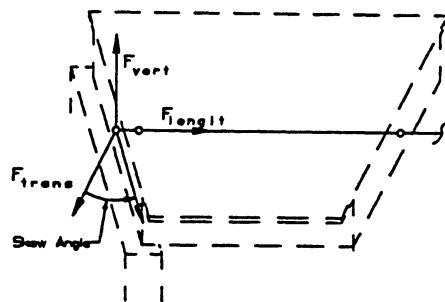
c) Abutment Displacement (or Force)
Coordinate Directions

Figure 1
BAG Abutment Release Conditions

SPAN DATA

[illegible]

Span lengths (ft.) along the centerline of bridge must be given for each span.

Moments of Inertia (ft.*4) of the superstructure must be given for span 1 either from this panel or calculate from the data in SPAN DATA PANEL - 2. The term "normal bending" is used to denote bending caused by vertical loads. For spans 2 through 48 if no value is given, BAG will assume the value from the previous span. Changes in moment of inertia may be designated by providing a new moment of inertia.

The cross sectional area (ft.**4) of the superstructure must be given for span 1 either from this panel or calculate from the data in SPAN DATA PANEL - 2. This area is used to compute the

superstructure weights. For spans 2 through 48 if no value is given, BAG will assume the value from the previous span. Changes in cross sectional area may be designated by providing a new cross sectional area.

SPAN MODULUS OF ELASTICITY

If this field is left blank, a modulus of elasticity of 468,000 ksf for concrete is assumed for the superstructure.

STRUCTURE DEPTH

Enter the superstructure depth of the bridge. The structure depth is used for wind on live load, longitudinal force, centrifugal force, and column end joint size. If zero structure depth is given, the moments generated by these forces are applied 6 feet above the bridge deck. If a structure depth other than zero is given, the generated moments are applied a distance $(6' + 1/2 \text{ structure depth})$ above the C.G. of the structure and column will have a end joint size of $1/2$ structure depth at the top. The design engineer may apply the moments created by wind on LL, longitudinal force, and centrifugal force about any axis he prefers by modifying the structure depth accordingly. If a structure has more than one depth entered, the structure depth used at the column will be the larger depth of the two adjacent spans.

ADDITIONAL MASS

Enter any masses which might add to the superstructure weight in addition to the cross section of the structure. This includes future wearing surface, barrier rail, and sidewalk. No default value.

SUPERSTRUCTURE SECTION PROPERTIES

[illegible]

Enter superstructure width from edge to edge. If superstructure width and structure depth on SPAN DATA PANEL - 1 are missing, no superstructure section property will be calculated.

Enter slab thickness of top slab.

BOTTOM SLAB THICKNESS

Enter slab thickness of bottom slab. Enter 0 in this field if it is T girder.

INTERIOR GIRDER**NUMBER OF INTERIOR GIRDER**

Enter number of interior girder. Enter 0 for single cell box girder.

WEB WIDTH OF INTERIOR GIRDER

Enter web width for interior girder only.

EXTERIOR GIRDER**TYPE**

Enter exterior girder type, 0 for vertical girder, 1 for sloped girder, 2 for sloped girder with 45 degrees outside slope, 3 for rounded girder. See FRAME SYSYEM manual for detail.

WIDTH

Enter exterior girder web width.

FACTOR

Enter horizontal distance from end of overhang to begining of bottom slab.

OVERHANG**LENGTH**

Enter horizontal distance from edge of deck to end of overhang.

EXTERIOR THICKNESS

Enter overhang thickness at edge of deck.

INTERIOR THICKNESS

Enter overhang thickness at end of overhang.

SPAN DATA (Continue)

[illegible]

HINGE LOCATION

HINGE CROSS SECTION

- 21 -

HINGE LENGTH

The hinge length in feet is measured along the centerline of the hinge in transverse direction of the bridge along the hinge diaphragm.

***** The corss sectional area and the hinge length are used to calculate the mass of the hinge in lbms/cu. ft.. BAG will evenly distribute the mass of the hinge to the two adjacent hinge joints.

RESTRAINER**RESTRAINER LENGTH**

The user may specify the restrainer length in feet. If the length is left blank, a length of 20 feet is assumed for the default value.

AREA

The user must specify the cross-sectional area of the restrainers in square inches. If the area is left blank or a zero restrainer area is input, no restrainer will be generated through that hinge.

RESTRAINER MODULUS OF ELASTICITY

A default value of 1,440,000 ksf (i.e., 10,000,000 psi for a steel cable) is assumed unless a value has been input, in which case it will override the default value.

NOTE: The restrainer generated is a STRUDL space truss member. The restrainer member spans the length of three superstructure members (i.e., the one foot long hinge member and two adjacent superstructure members). Therefore, the area of the restrainer is adjusted within the program so that the restrainer's elastic stiffness will be consistent with the specified length of the restrainer. This is done as follows:

AREA for analysis = Area input X Length for analysis
divided by the input length.

WIND LOAD

The user may specify the wind intensity on the bridge (kips/feet). The default value is 1.0.

WIND ON LIVE LOAD

The user may specify the wind intensity on live load (kips/feet). The default value is 0.1.

LIVE LOAD LOGITUDINAL FORCE

The user may specify the logitudinal force caused by live load (kips/feet). The default value is 1.0.

BENT DATA

[illegible]

Specify the elevation in feet at both the center of gravity of the superstructure and the bottom of column. Both values must be entered. A zero length column (i.e., difference in elevations = zero) is not permitted. The elastic column length is computed as the difference between these elevations. These elevations must be consistent with the abutment elevations.

Enter the bearing (Deg,Min,Sec) of the centerline of bent. The bearing may be specified in either direction. When no bent bearing is given it will be assumed to be normal to the superstructure centerline (radial on curved alignment). Bearings resulting in 90 degree skews (i.e., bents parallel to the superstructure) are not permitted.

NUMBER OF COLUMNS IN BENT

The user must specify the number of columns in the bent. A maximum of 99 columns per bent is permitted.

COLUMN SPACING

This entry is required only for bents having more than one column. Specify the distance in feet between the column centerlines along the centerline of the bent. Specifying a column spacing for a single column bent will cause the problem to be rejected.

COLUMN MODULUS OF ELASTICITY

Enter the modulus of elasticity (ksf) of the concrete column. If this field is left blank, a default modulus of 468,000 ksf is used.

COLUMN TYPE

Specify the column type numbers to be recalled for use in the bent. (See Column Data Panel) Note that only one column type may be used in each bent.

BENT CAP DATA**IZ**

Enter the moment of inertia (ft^4) for normal bending (i.e., in the plane of the bent caused by vertical loads) and the cross sectional area (ft^2). Note that the moments of inertia about the X and Y axes are assumed by the program. The moment of inertia about the X-axis (i.e., torsion) and the moment of inertia about the Y-axis (i.e., bending in a plane parallel to the superstructure deck) are assumed to be 1000 times IZ. Stiffness about these two axes is usually considerably larger due to the presence of the superstructure surrounding it. This effect is approximated by increasing the two moments of inertia by the factor given above.

AREA

Enter the cap cross sectional area.

LENGTH

Enter the cap length in transverse direction of the bridge along the cap.

BENT DATA (Continue)

Loads acting on the bent are described on this panel.

[illegible]

The user may enter the column wind forces or use the default values in the BAG program. The default value is 1.0 for the transverse direction and 0.1 for the longitudinal direction.

The user must enter the value of the centrifugal force since the BAG program does not provide a default value for this force. The BAG program calculates the moments associated with the centrifugal force using the structure depth previously described. If all six values are omitted, BAG will not generate these loading cases. Enter centrifugal forces by the different loading condition, i.e. the force associate with different live load conditions, max. longitudinal moment, max. tranverse moment, and max. axial force at the bent.

COLUMN RELEASES AT BOTTOM**FORCE RELEASES**

The user has control over all of the force releases at the bottom of column. The releases are applied in the plane of the bent and apply to all of the columns in the bent. Indicate the column fixity by specifying a zero (or blank) for a force release and a "1" for continuity (i.e., fixed condition). If the field is left entirely blank (i.e., all three are zeros), fixed conditions will be assumed for force at the column ends except at the axial direction which is not allowed to release. Note that a single non-zero entry overrides the default and requires a complete specification of all of the releases. In addition to condition specified above, user can input spring constant at bottom of column.

MOMENT RELEASES

The user has control over all of the moment releases at the bottom of column. The releases are applied in the plane of the bent and apply to all of the columns in the bent. Indicate the column fixity by specifying a zero (or blank) for a moment release and a "1" for continuity (i.e., fixed condition). Note that a single non-zero entry overrides the default and requires a complete specification of all of the releases. In addition to condition specified above, user can input spring constant at bottom of column.

Column Data

```
*****  
**                                     **  
**                                  BAG**  
**                                *****
```

| COLUMN DATA | | | | | | |
|-------------|-----|---------|-----------|------------|---------------------------|---------|
| STORE : | | SEGMENT | | | | |
| COLUMN: | | LENGTH | AREA | : | MOMENT OF INERTIA (FT**4) | |
| TYPE : | NO. | (FT) | (FT**2) : | IZ(LONGIT) | IY(TRANS) | IX(TOR) |
| - | -- | ---- | ----- | | ----- | ----- |
| - | -- | ---- | ----- | | ----- | ----- |
| - | -- | ---- | ----- | | ----- | ----- |
| - | -- | ---- | ----- | | ----- | ----- |
| - | -- | ---- | ----- | | ----- | ----- |
| - | -- | ---- | ----- | | ----- | ----- |
| - | -- | ---- | ----- | | ----- | ----- |
| - | -- | ---- | ----- | | ----- | ----- |
| - | -- | ---- | ----- | | ----- | ----- |
| - | -- | ---- | ----- | | ----- | ----- |
| - | -- | ---- | ----- | | ----- | ----- |
| - | -- | ---- | ----- | | ----- | ----- |
| - | -- | ---- | ----- | | ----- | ----- |
| - | -- | ---- | ----- | | ----- | ----- |
| - | -- | ---- | ----- | | ----- | ----- |
| - | -- | ---- | ----- | | ----- | ----- |

```
**                                *****
```

| | | | | | | | | | |
|--------|---------|---------|---------|---------|----------|-------|-------|---------|---------|
| F1/RUN | F2/GEOM | F3/QUIT | F4/SPAN | F5/BENT | F6/PSAVE | F7/PB | F8/PF | F9/FILE | F10/COL |
|--------|---------|---------|---------|---------|----------|-------|-------|---------|---------|

```
**                                *****
```

Specify the store column type number. A maximum of 10 types (0-9) are permitted for a bridge. The store column types are numbered 0 through 9 and must be identical for all segments of each column.

Segment numbers must be numbered sequentially starting with number 1 at the bottom of the column. A maximum of 20 column segments may be used to describe a column.

The segment length is not required for a single segment column. Only one zero length segment (i.e., blank) is permitted for a

non-prismatic column. The zero length segment will be used to adjust the overall column length to conform to the elevations specified on the Bent Data Card. If no zero length segment is specified or total of segments length is less than the column length, the first segment at the bottom of the column will be adjusted. The length of all other segments must be specified.

SEGMENT AREA

Specify the cross-sectional area (ft**2) of each prismatic segment for each column type.

SEGMENT MOMENT OF INERTIA

Specify the moments of inertia (ft**4) of each prismatic segment for each column type. These are referenced relative to the plane of the bent.

Note: The "STORE COLUMN TYPE" number must correspond to the "COLUMN TYPE" number specified for recall on the BENT DATA PANEL. These numbers can vary from 0 to 9 and only one column type is permitted in any one bent. All described column types do not have to be recalled by the bent data.

GENERATED OUTPUT

GENERATION NUMBERING CONVENTION

BAG generates joints and members according to a predetermined numbering convention. This makes it easy for the user to interpret the STRUDL output.

JOINTS

Joint numbers are either four or five digits long. Joint numbers are determined by the location of the joints within the bridge model. Table 3.1 defines the significance of the joint numbers.

| Joint | Digit Number | | | |
|----------------|--------------|-------------|-----------|-----------|
| Location | 1 | 2 & 3 | 4 | 5 |
| SUPERSTRUCTURE | 1 | SPAN NO. | JOINT NO. | -- |
| HINGE | 2 | SPAN NO. | JOINT NO. | -- |
| ABUTMENT | 3 | SUPPORT NO. | JOINT NO. | -- |
| COLUMN | 4 | BENT NO. | COL. NO. | JOINT NO. |

Table 3.1 - Joint Numbering Convention

MEMBERS

Members are numbered using a similar convention. Table 3.2 defines the member numbering convention.

| Joint | Digit Number | | | |
|----------------|--------------|----------|----------|----------|
| Location | 1 | 2 & 3 | 4 | 5 |
| SUPERSTRUCTURE | 5 | SPAN NO. | MEM. NO. | -- |
| CAP | 6 | BENT NO. | MEM. NO. | -- |
| COLUMN | 7 | BENT NO. | COL. NO. | MEM. NO. |
| RESTRAINER | 8 | SPAN NO. | -- | -- |

Table 3.2 - Member Numbering Convention

GENERATED STRUDL OUTPUT REQUESTS

The following results are output from the generated STRUDL analysis.

1. FREQUENCIES OF ALL MODES OF VIBRATION
2. Normalized eigenvectors (mode shapes) for the number of modes included in the response spectrum analysis.
3. Dynamic participation factors for the number of modes requested.
4. The square root of the sum of the squares (RMS) of the response spectrum forces and moments at the base and top of all columns for Case 1 and Case 2.
5. The response spectrum RMS forces in the restrainers for Case 1 and Case 2.
6. The response spectrum RMS displacements of the deck joints located at the abutments, bents, and hinges for Case 1 and Case 2.
8. The modal shock spectrum result force and displacement at location of the above items 4-7 for load case in X and Z directions.
7. The response spectrum RMS forces and moments in the deck at abutments and hinges for Case 1 and Case 2.

INPUT DATA FORMAT (XEDIT)

RUN DATA

| (I2,7X,6A4,I3,2I1,I3,I2,F5.4,5I1,1X,2F9.2,I2,I1) | | | Variable |
|--|--------------------------------|--|----------|
| Columns | Entry | | |
| 1 - 2 | CARD TYPE - 1 | | IN(1) |
| 3 - 9 | BLANK | | |
| 10 - 33 | COMMENTS | | NCOM |
| 34 - 36 | NO. OF MODES | | NOM |
| 37 | NO. OF EXTRA JOINTS PER SPAN | | NJS |
| 38 | NO. OF EXTRA JOINTS PER COLUMN | | NJC |
| 39 - 41 | MAXIMUM NO. OF JOINTS | | JMAX |
| 42 - 43 | MAXIMUM NO. OF SPANS | | NSPX |
| 44 - 48 | SPECTRUM MODIFICATION FACTOR | | SF |
| 49 | PLOT CONTROL | | NPLT |
| 50 | CONDENSE CONTROL | | NCND |
| 51 | REDUCED INPUT _ OUTPUT DATA | | NRIOD |
| 52 | DYNAMIC AND STATIC OUTPUT | | NRSAD |
| 53 | ROTATION CONTROL | | NTEST |
| 55 - 63 | NORTH COORDINATE | | XNC |
| 64 - 72 | EAST COORDINATE | | XECO |
| 73 - 74 | BEGINNING ABUTMENT NUMBER | | NA1 |
| 75 | PROGRAM TYPE | | NTYPE |

GEOMETRY DATA

| (I2,7X,I1,A1,3I2,A1,2(A1,F7.2,F8.2,A1,3I2,A1),F8.2) | | | Variable |
|---|------------------------------|--|----------|
| Columns | Entry | | |
| 1 - 2 | CARD TYPE - 2 | | IN(2) |
| 3 - 9 | BLANK | | |
| 10 | NO. OF CURVES | | NC |
| 11 | N/S | | IQA(1) |
| 12 - 13 | DEGREES | | XDG(1) |
| 14 - 15 | MINUTES | | XM(1) |
| 16 - 17 | SECONDS | | XS(1) |
| 18 | E/W | | IQB(1) |
| 19 | DIRECTION OF CURVE | | ND(1) |
| 20 - 26 | RADIUS OF CURVE 1 (ft.) | | RAD(1) |
| 27 - 34 | BEGINNING OF CURVE STA (ft.) | | BC(1) |
| 35 | N/S | | IQA(2) |
| 36 - 37 | DEGREES | | XDG(2) |
| 38 - 39 | MINUTES | | XM(2) |
| 40 - 41 | SECONDS | | XS(2) |
| 42 | E/W | | IQB(2) |
| 43 | DIRECTION OF CURVE | | ND(2) |

| | | |
|---------|--------------------------|--------|
| 44 - 50 | RADIUS OF CURVE 2 (ft.) | RAD(2) |
| 51 - 58 | BEGINNING OF CURVE STA | BC(2) |
| 59 | N/S | IQA(3) |
| 60 - 61 | DEGREES | XDG(3) |
| 62 - 63 | MINUTES | XM(3) |
| 64 - 65 | SECONDS | XS(3) |
| 66 | E/W | IQB(3) |
| 67 - 74 | ROTATION ANGLE (degrees) | ROTA |

ABUTMENT DATA

| (I2,7X,F7.2,(F8.2,A1,3I2,A1,2F6,2I1)) | | Variable |
|---------------------------------------|----------------------------------|------------|
| Columns | Entry | |
| 1 - 2 | CARD TYPE - 3 | IN(3) |
| 3 - 9 | BLANK | |
| 10 - 16 | BEGINNING ABUT. STATION (ft.) | A1S |
| 17 - 21 | BEGINNING ABUT. ELEVATION (ft.) | BT(1) |
| 22 | N/S | N(1) |
| 23 - 24 | DEGREES | D(1) |
| 25 - 26 | MINUTES | XM(1) |
| 27 - 28 | SECONDS | S(1) |
| 29 | E/W | M(1) |
| | BEGINNING ABUTMENT RELEASE ARRAY | |
| 30 - 35 | LONGITUDINAL | IAR(1,1) |
| 36 - 41 | TRANSVERSE | IAR(2,1) |
| 42 | VERTICAL | IAR(3,1) |
| 43 | TORSIONAL MOMENT | IAR(6,1) |
| 44 - 48 | END ABUTMENT ELEVATION (ft.) | BT(IN\$+1) |
| 49 | N/S | N(2) |
| 50 - 51 | DEGREES | D(2) |
| 52 - 53 | MINUTES | XM(2) |
| 54 - 55 | SECONDS | S(2) |
| 56 | E/W | M(2) |
| | END ABUT. RELEASE ARRAY | |
| 57 - 62 | LONGITUDINAL | IAR(1,2) |
| 63 - 68 | TRANSVERSE | IAR(2,2) |
| 69 | VERTICAL | IAR(3,2) |
| 70 | TORSIONAL MOMENT | IAR(6,2) |

SPAN DATA - 1

| (I2,7X,I2,F4.1,F6.1,F7.1,F6.1,3F5.1,F2,F6.1,F4,2F4.1,F4.2) | | Variable |
|--|---------------|----------|
| Columns | Entry | |
| 1 - 2 | CARD TYPE - 4 | IN(4) |
| 3 - 9 | BLANK | |
| 10 - 11 | SPAN NUMBER | N |

| | | |
|---------|--|---------|
| 12 - 15 | SPAN LENGTH (ft.) | SP(N) |
| 16 - 21 | NORMAL BENDING IZ (ft.**4) | SIZ(N) |
| 22 - 28 | TRANSVERSE BENDING IY (ft.**4) | SIY(N) |
| 29 - 34 | TORSIONAL IX (ft.**4) | SIX(N) |
| 35 - 39 | CROSS_SECTIONAL AREA AX (ft.**2) | SAX(N) |
| 40 - 44 | SPAN MODULUS OF ELASTICITY (10**3 ksf) | SMOD(N) |
| 45 - 49 | HINGE LOCATION (FROM LEFT END OF SPAN, ft.) | XH(N) |
| 50 - 51 | RESTRAINER LENGTH (ft.) | RL(N) |
| 52 - 57 | RESTRAINER AREA (in.**2) | AR(N) |
| 58 - 61 | RESTRAINER MODULUS OF ELASTICITY (10**3 ksf) | RMOD(N) |
| 62 - 65 | HINGE CROSS_SECTION AREA AX (ft.**2) | AH(N) |
| 66 - 69 | HINGE LENGTH (ft.) | HL(N) |
| 70 - 73 | STRUCTURE DEPTH (ft.) | XD(N) |

SPAN DATA - 2

| Columns | Entry | Variable |
|---|--------------------------------------|----------|
| (I2,7X,I2,2F3.1,F4.1,2F4.2,I2,F2,2(I1,F2,F3.2),2(F2.1,2F2)) | | |
| 1 - 2 | CARD TYPE - 5 | IN(5) |
| 3 - 9 | BLANK | |
| 10 - 11 | SPAN NUMBER | N |
| 12 - 17 | BLANK | |
| 18 - 21 | Superstructure width | W |
| 22 - 25 | Top slab thickness | TSUBT |
| 26 - 29 | Bottom slab thickness | TSUBB |
| 30 - 31 | Number of interior girders | NOGIR |
| 32 - 33 | Interior girder width | TSUBW |
| 34 - 34 | Left exterior girder type | ITYPE |
| 35 - 36 | Left exterior girder width | XLWUBW |
| 37 - 39 | Left exterior girder slope factor | XGF(1) |
| 40 - 40 | Right exterior girder type | JTYPE |
| 41 - 42 | Right exterior girder width | XRWUBW |
| 43 - 45 | Right exterior girder slope factor | XGF(2) |
| 46 - 47 | Left overhang length | OHL |
| 48 - 49 | Exterior thickness of left overhang | TEXTL |
| 50 - 51 | Interior thickness of left overhang | TINTL |
| 52 - 53 | Right overhang length | OHL |
| 54 - 55 | Exterior thickness of right overhang | TEXTL |
| 56 - 57 | Interior thickness of right overhang | TINTL |

BENT DATA - 1

| Columns | Entry | Variable |
|---|---------------|----------|
| (I2,7X,I2,2F5.1,A1,3F2,A1,I2,F5.2,F5,I2,F5.1,F4.1,F6.2) | | |
| 1 - 2 | CARD TYPE - 6 | IN(6) |

| | | |
|---------|---|---------|
| 3 - 9 | BLANK | |
| 10 - 11 | BENT NUMBER | N |
| 12 - 16 | ELEVATION TOP OF BENT (ft.) | BT(N) |
| 17 - 21 | ELEVATION FOOTING (ft.) | BF(N) |
| 22 | N/S | ND(1) |
| 23 - 24 | DEGREES | DEG |
| 25 - 26 | MINUTES | XM |
| 27 - 28 | SECONDS | SEC |
| 29 | E/W | ND(2) |
| 30 - 31 | NUMBER OF COLUMNS IN BENT | NCOL(N) |
| 32 - 36 | C-C COLUMN SPACING (ft.) | CC(N) |
| 37 - 41 | COLUMN MODULUS OF ELASTICITY (10**3 ksf) | CMOD(N) |
| 42 - 43 | RECALL COL TYPE | NCT(N) |
| 44 - 48 | MOMENT OF INERTIA (IZ) OF BENT CAP (ft.**4) | CI(N) |
| 49 - 52 | CROSS SECTIONAL AREA OF BENT CAP (ft.**2) | CA(N) |
| 53 - 58 | BENT CAP LENGTH (ft.) | CAPL(N) |

BENT DATA - 2

| (I2,7X,I2,3I1,6I8) | | |
|--------------------|----------------------------|----------|
| Columns | Entry | Variable |
| 1 - 2 | CARD TYPE - 7 | IN(7) |
| 3 - 9 | BLANK | |
| | COLUMN MOMENT RELEASES TOP | |
| 10 | LONGITUDINAL | MRT(1,N) |
| 11 | TRANSVERSE | MRT(2,N) |
| 12 | TORSIONAL | MRT(3,N) |
| | COLUMN RELEASES BOTTOM | |
| | FORCE RELEASES | |
| 13 - 20 | LONGITUDINAL | IRB(1,N) |
| 21 - 27 | TRANSVERSE | IRB(2,N) |
| 28 - 35 | AXIAL | IRB(3,N) |
| | MOMENT RELEASES | |
| 36 - 43 | LONGITUDINAL | IRB(4,N) |
| 44 - 51 | TRANSVERSE | IRB(5,N) |
| 52 - 59 | TORSIONAL | IRB(6,N) |

COLUMN DATA

| (I2,7X,I2,I1,I2,F6,F5,3F6) | | |
|----------------------------|----------------------------|----------|
| Columns | Entry | Variable |
| 1 | CARD TYPE - 8 | IN(8) |
| 2 - 9 | BLANK | |
| 10 | STORE COLUMN TYPE | J |
| 11 - 12 | SEGMENT NUMBER FROM BOTTOM | M |

| | | |
|---------|------------------------------------|------------|
| 13 - 16 | SEGMENT LENGTH (ft.) | SL(J+1,M) |
| 17 - 21 | SEGMENT AREA (ft.**2) | SA(J+1,M) |
| | SEGMENT MOMENT OF INERTIA (ft.**4) | |
| 22 - 27 | LONGITUDINAL (IZ) | SIL(J+1,M) |
| 28 - 33 | TRANSVERSE (IY) | SIT(J+1,M) |
| 34 - 39 | TORSIONAL (IX) | SIX(J+1,M) |

EARTHQUAKE DATA

(I2,7X,2A1,6(A1,3F2,A1))

| Columns | Entry | Variable |
|---------|----------------------------------|----------|
| 1 - 2 | CARD TYPE - 9 | IN(9) |
| 3 - 9 | BLANK | |
| 10 | PEAK ROCK ACCELERATION | PEAK |
| 11 | CURVE DESIGNATION | NCUR |
| | ADDITIONAL DIRECTIONS (MAX. SIX) | |
| 12 | N/S | N(I) |
| 13 - 14 | DEGREES | D(I) |
| 15 - 16 | MINUTES | XM(I) |
| 17 - 18 | SECONDS | S(I) |
| 19 | E/W | M(I) |
| 62 - 69 | SPECIAL SITE SPECTRUM | SPEC |

SPAN DATA - 3

(I2,7X,I2,4F4)

| Columns | Entry | Variable |
|---------|-----------------------------|----------|
| 1 - 2 | CARD TYPE - 10 | IN(10) |
| 3 - 9 | BLANK | |
| 10 - 11 | SPAN NUMBER | N |
| 12 - 15 | WIND LOAD (klf) | SWN(N) |
| 16 - 19 | WIND ON LIVE LOAD (klf) | SWL(N) |
| 20 - 23 | ADDITIONAL MASS (lbm) | MASS(n) |
| 24 - 28 | TEMPERATURE RANGE (degrees) | TEMP |

BENT DATA - 3

(I2,7X,I2,8F6)

| Columns | Entry | Variable |
|---------|----------------|----------|
| 1 - 2 | CARD TYPE - 11 | IN(11) |

| | | |
|---------|--|---------|
| 3 - 9 | BLANK | |
| 10 - 11 | BENT NUMBER | N |
| 12 - 17 | WIND LOAD ON COLUMN (klf) | SWCT(N) |
| 18 - 23 | WIND LOAD ON COLUMN (klf) | SWCL(N) |
| 24 - 29 | CENTRIFUGAL FORCE H LOAD CASE 1 (kips) | CFH1(N) |
| 30 - 35 | CENTRIFUGAL FORCE H LOAD CASE 2 (kips) | CFH2(N) |
| 36 - 41 | CENTRIFUGAL FORCE H LOAD CASE 3 (kips) | CFH3(N) |
| 42 - 47 | CENTRIFUGAL FORCE P LOAD CASE 1 (kips) | CFP1(N) |
| 48 - 53 | CENTRIFUGAL FORCE P LOAD CASE 2 (kips) | CFP2(N) |
| 54 - 59 | CENTRIFUGAL FORCE P LOAD CASE 3 (kips) | CFP3(N) |

EXAMPLE PROBLEM

EXAMPLE 1

PROBLEM STATEMENT

Generate BAG input coding to analyze the three span structure shown in Figure 2 using the given properties.

Superstructure Properties

| | | |
|-------------------------|----------|----------------------|
| Span length | - span 1 | 80 ft |
| | span 2 | 100 ft |
| | span 3 | 80 ft |
| Total length | | 260 ft |
| Cross-sectional area | | 61 ft ² |
| Moments of inertia | | |
| Torsion (IX) | | 809 ft ⁴ |
| Normal Bending (IZ) | | 510 ft ⁴ |
| Transverse Bending (IY) | | 6200 ft ⁴ |
| Modulus of Elasticity | | 468000 ksf |

Substructure Properties

Bent 2

| | |
|-------------------|----------|
| Top Elevation | 207.0 ft |
| Bottom Elevation | 183.0 ft |
| Number of columns | 1 |

Column

| | |
|----------------------|----------------------|
| Cross-sectional area | 15.0 ft ² |
| Moments of inertia | |
| Torsion (IX) | 45.0 ft ⁴ |
| Normal Bending (IZ) | 11.2 ft ⁴ |

| | |
|-----------------------------|-----------------------|
| Transverse Bending (IY) | 31.2 ft ⁴ |
| Modulus of Elasticity | 468000.0 ksf |
| Bent 3 | |
| Top Elevation | 203.2 ft |
| Bottom Elevation | 148.5 ft |
| Number of columns | 2 |
| Column Spacing | 32.0 ft |
| Column | |
| Cross-sectional area | 12.0 ft ² |
| Moments of inertia | |
| Torsion (IX) | 36.0 ft ⁴ |
| Normal Bending (IZ) | 9.0 ft ⁴ |
| Transverse Bending (IY) | 16.0 ft ⁴ |
| Modulus of Elasticity | 468000.0 ksf |
| Cap | |
| Cross-sectional area | 40.0 ft ² |
| Normal Bending (IZ) | 700.0 ft ⁴ |
| Modulus of Elasticity | 468000.0 ksf |
| Hinge | |
| Location | 20.0 ft from bent 2 |
| Length | 24.0 ft |
| Cross-sectional area | 40.0 ft ² |
| Hinge Restrainer Properties | |
| Length | 20.0 ft |
| Cross-sectional area | 3.1 in ² |
| Modulus of Elasticity | 1440000.0 ksf |

Seismic Loading (CALTRANS)

| | |
|-------------------------------|---------|
| Peak rock acceleration | 0.6 g |
| Depth to "rock-like" material | 0-10 ft |

Horizontal Alignment

Curve 1

| | |
|----------------------|-------------|
| Tangent Bearings | |
| BC to PI | N83 29'40"E |
| PI to EC | S61 40'25"E |
| Radius | 300 ft |
| Station BC | 10+25.10 |
| Station Abutment 1 | 11+05.10 |
| Elevation Abutment 1 | 210.20 |

Curve 2

| | |
|----------------------|-------------|
| Tangent Bearings | |
| BC to PI | S61 40'25"E |
| PI to EC | N79 29'30"E |
| Radius | 300 ft |
| Station BC | 12+75310 |
| Elevation Abutment 4 | 200.20 |

BAG Panel Input

Input Data File

```

|...+...1...+...2...+...3...+...4...+...5...+...6...+...7...+...8
1      EXAMPLE PROBLEM                      32  481.0001                      1
2      2N832940ER      300  102510S614025EL      300  127530N792930E      0
3      11051 2101      0      011 2001N342945E      300  10011
4      1 800  5100  62000  8090  610  0      0 0      0 0 0 0 500
4      21000  5100  62000  8090  610  0      20020      31  0 400 240  0
4      3 800      0      0      0 0 0 0      0 0      0 0 0 0
5      1      0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
5      2      0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
5      3      800 800 600 515012 0012 060 71550 715
6      2 2070 1830      1 0      0 1 7000 400 3200
6      3 2032 1485N 93414W 2 2400      0 2 7000 400 3200
7      1 1 0 150 112 312 450
7      2 1 0 120 90 160 360
8      2000      0      0      0      0      0      0
8      3000      0      0      0      0      0      0
9      6A      0      0      0      0      0      0
10     1 0 0 0 0 0 0
10     2 0 0 0 0 0 0
10     3 0 0 0 0 0 0
11     2 0 0 0 0 0 0 0 0 0
11     3 0 0 0 0 0 0 0 0

```

BAG Panel Input Screen 1 - Input

```

*****
*
*
*
*          BAG          V 3.01 RELEASE MAY 18, 1988
*
* RUN DATA
*
* COMMENTS ==> example problem
* NOM NJS NJC JMAX NSPX SF PLOT ANALYSIS ROTATION REDUCE I/O CONDEN.
*   3  2   48  1   1   -   -   -   -
*
* NOM : NO. OF MODES      ( DEFAULT = 12 OR 3 * NO OF SPANS )
* NJS : NO. OF EXTRA JOINTS ADDED PER SPAN ( DEFAULT = 3 )
* NJC : NO. OF EXTRA JOINTS ADDED PER COL. ( DEFAULT = 2 )
* JMAX: MAX. NO. OF JOINTS      ( DEFAULT = 75 )
* NSPX: MAX. NO. OF SPANS      ( DEFAULT = 48 )
* PLOT: PLOT LENGTH CODE ( LENGTH = 18 + 6 * PLOT INCHES, 0 FOR NO PLOT)
* ANALYSIS: 0 = STATIC & DYNAMIC , 1 = DYNAMIC ONLY , 2 = STATIC ONLY
* ROTATION: 0 = YES          , 1 = NO
*
*-----
* EARTHQUAKE LOAD DATA
* PEAK :CURVE: UP TO 6 ADDITIONAL OPTIONAL DIRECTIONS FOR GIVEN LOAD
* ROCK : DES.: : : : : : :
* ACC. : : : 2 : 3 : 4 : 5 : 6 : 7
* 1/10 G: A-D : : : : : : :
* 6 A
* SPECIAL SITE SPECTRUM (8 CHARACTERS NAME FOR STRUDL)
*
*****
* F1/RUN F2/GEOM F3/QUIT F4/SPAN F5/BENT F6/PSAVE F7/PB F8/PF F9/FILE F10/COL
*

```

BAG Panel Input Screen 1 - Output

```

*****
*
*
*
*          BAG          V 3.01 RELEASE MAY 18, 1988
*
* RUN DATA
*
* COMMENTS ==> EXAMPLE PROBLEM
* NOM NJS NJC JMAX NSPX SF PLOT ANALYSIS ROTATION
*   3  2   48  1.0000 1
*
* NOM : NO. OF MODES      ( DEFAULT = 12 OR 3 * NO OF SPANS )
* NJS : NO. OF EXTRA JOINTS ADDED PER SPAN ( DEFAULT = 3 )
* NJC : NO. OF EXTRA JOINTS ADDED PER COL. ( DEFAULT = 2 )
* JMAX: MAX. NO. OF JOINTS      ( DEFAULT = 75 )
* NSPX: MAX. NO. OF SPANS      ( DEFAULT = 48 )
* PLOT: PLOT LENGTH CODE ( LENGTH = 18 + 6 * PLOT INCHES, 0 FOR NO PLOT)
* ANALYSIS: 0 = STATIC & DYNAMIC , 1 = DYNAMIC ONLY , 2 = STATIC ONLY
* ROTATION: 0 = YES          , 1 = NO
*
*-----
* EARTHQUAKE LOAD DATA
* PEAK :CURVE: UP TO 6 ADDITIONAL OPTIONAL DIRECTIONS FOR GIVEN LOAD
* ROCK : DES.: : : : : : :
* ACC. : : : 2 : 3 : 4 : 5 : 6 : 7
* 1/10 G: A-D : : : : : : :
* 6 A
* SPECIAL SITE SPECTRUM (8 CHARACTERS NAME FOR STRUDL)
*
* F1/RUN F2/GEOM F3/QUIT F4/SPAN F5/BENT F6/PSAVE F7/PB F8/PF F9/FILE F10/COL
*
*****

```

```
*****
```

| GEOMETRY DATA | | | | | | | |
|--|------------|-----------|-----------------|------------------|-----------|------------------------|----------|
| ORIGIN DATA | | | | | | TANGENT OR BC TO PI | ROTATION |
| COORDINATES | | | | BEARING | ANGLE | | |
| Z | X | | n 832940 e | (DEGREES) | | | |
| CURVE DATA | | | | | | | |
| CURVE NO. | DIR R/L | RADIUS FT | BC FT | BEARING PI TO EC | | | |
| 1 | r | 300 | 1025.1 | s 614025 e | | | |
| 2 | l | 300 | 1275.3 | n 792930 e | | | |
| ----- | | | | | | | |
| ABUTMENT DATA | | | | | | | |
| ABUTMENT STATION : ELEVATION : | BEARING : | | RELEASE FORCE : | | MOMENT | | |
| FT : FT : | : | | : | : LONG TRANS V | TORSIONAL | | |
| 1_ 1105.1_ 210.1_ | n 342945 e | | 300_ | 100_ | 1 | 1 | |
| _ 200.1_ | | | | | 1 | 1 | |
| DESIGN TEMPERATURE RANGE (F) : _ DEGREES | | | | | | | |
| (TEMPERATURE COEFFICIENT IS BASED ON CONCRETE. USER SHOULD CHECK STRUDL CODE.) | | | | | | | |
| F1/RUN F2/GEOM F3/QUIT F4/SPAN F5/BENT F6/PSAVE F7/PB F8/PF F9/FILE F10/COL | | | | | | | |

```
*****
```

```

*****
          BAG
GEOMETRY DATA
ORIGIN DATA
      COORDINATES
            Z              X
            0.00         0.00
TANGENT OR BC TO PI BEARING N 832940 E
ROTATION ANGLE (DEGREES) 0.0000
CURVE DATA
      CURVE NO.   DIR    RADIUS FT    BC FT    BEARING PI TO EC
           1       R     300    1025.10 S 614025 E
           2       L     300    1275.30 N 792930 E
-----
ABUTMENT DATA
ABUTMENT STATION :ELEVATION: BEARING : RELEASE FORCE : MOMENT
             FT : FT : : LONG TRANS V : TORSIONAL
           1    1105.1    210.1    - 0        0    1    1
           1    200.1    200.1    N 342945 E    300    100    1    1
DESIGN TEMPERATURE RANGE (F) : 0 DEGREES
(TEMPERATURE COEFFICIENT IS BASED ON CONCRETE. USER SHOULD CHECK STRUDL CODE.)
F1/RUN F2/GEOM F3/QUIT F4/SPAN F5/BENT F6/PSAVE F7/PB F8/PF F9/FILE F10/COL
*****

```


BAG Panel Input Screen 4 - Input

```

*****
*
*
*
*          BAG
*
*  SPAN DATA -2
*
*          : SUPERSTRUCTURE:  SLAB DATA  : INT. GIRD.
*          SPAN : DATA : TOP    BOT :      WEB
*          NO. : WIDTH : THICK  THICK :      THICK.
*          :      (FT) : (IN)   (IN) :      NUM   (IN)
*          1      0.0   0.00   0.00   0      0
*          2      0.0   0.00   0.00   0      0
*          3     80.0   8.00   6.00   5     15
*
*          ---
*          ---
*          ---
*
*          : EXTERIOR GIRDERS : OVERHANGS
*          SPAN: LEFT : RIGHT : LEFT : RIGHT
*          NO.: W.TH. FACT. : W.TH. FACT. : LEN. E.TH. I.TH. : LEN. E.TH. I.TH.
*          :TYPE (IN) (FT) :TYPE (IN) (FT) : (FT) (IN) (IN) : (FT) (IN) (IN)
*          1  0      0 0.00  0      0 0.00  0.0 0      0      0.0 0      0
*          2  0      0 0.00  0      0 0.00  0.0 0      0      0.0 0      0
*          3  0     12 0.00  0     12 0.00  6.0 7     15     5.0 7     15
*
*          ---
*          ---
*          ---
*
*          F1/RUN F2/GEOM F3/QUIT F4/SPAN F5/BENT F6/PSAVE F7/PB F8/PF F9/FILE F10/COL
*
*****

```

BAG Panel Input Screen 4 - Output

```

*****
*
*
*
*          BAG
*
*  SPAN DATA -2
*
*          : SUPERSTRUCTURE:  SLAB DATA  : INT. GIRD.
*          SPAN : DATA : TOP    BOT :      WEB
*          NO. : WIDTH : THICK  THICK :      THICK.
*          :      (FT) : (IN)   (IN) :      NUM   (IN)
*          1      0.0   0.00   0.00   0      0
*          2      0.0   0.00   0.00   0      0
*          3     80.0   8.00   6.00   5     15
*
*          ---
*          ---
*          ---
*
*          : EXTERIOR GIRDERS : OVERHANGS
*          SPAN: LEFT : RIGHT : LEFT : RIGHT
*          NO.: W.TH. FACT. : W.TH. FACT. : LEN. E.TH. I.TH. : LEN. E.TH. I.TH.
*          :TYPE (IN) (FT) :TYPE (IN) (FT) : (FT) (IN) (IN) : (FT) (IN) (IN)
*          1  0      0 0.00  0      0 0.00  0.0 0      0      0.0 0      0
*          2  0      0 0.00  0      0 0.00  0.0 0      0      0.0 0      0
*          3  0     12 0.00  0     12 0.00  6.0 7     15     5.0 7     15
*
*          ---
*          ---
*          ---
*
*          F1/RUN F2/GEOM F3/QUIT F4/SPAN F5/BENT F6/PSAVE F7/PB F8/PF F9/FILE F10/COL
*
*****

```

```

*****
*
*
*
*
*          BAG
*
*    SPAN DATA - 3
*
*   SPAN :----- HINGE -----:----- RESTRAINER-----: WIND   WIND   LL
*   NO.  :LOCATION X-SECTION LENGTH : LENGTH AREA     E  : LOAD   ON LL   LF
*         : (FT)   (FT**2)  (FT) : (FT)  (IN**2) (KSF) : (KLF)  (KLF)  (KLF)
*   1      0.0       0.00    0.0   0.0   0.0        0      0      0
*   2     20        40       24    20    3.1         0      0      0
*   3      0.0       0.00    0.0   0.0   0.0         0      0      0
*   4      _____
*   5      _____
*   6      _____
*   7      _____
*   8      _____
*   9      _____
*  10      _____
*  11      _____
*  12      _____
*  13      _____
*  14      _____
*  15      _____
*  16      _____
*
* F1/RUN F2/GEOM F3/QUIT F4/SPAN F5/BENT F6/PSAVE F7/PB F8/PF F9/FILE F10/COL
*
*****

```

```

*****
#
#
#
#
#                                     BAG
#
#       SPAN DATA - 3
#
#   SPAN :----- HINGE -----:----- RESTRAINER-----: WIND    WIND    LL
#   NO.  :LOCATION X-SECTION LENGTH : LENGTH AREA     E : LOAD ON LL LF
#         : (FT)   (FT**2)      (FT) : (FT) (IN**2) (KSF) : (KLF) (KLF) (KLF)
#   1      0.0        0.00        0.0    0.0    0.0      0  0.000  0.000  0.000
#   2      20.0       40.00       24.0   20.0    3.1      0  0.000  0.000  0.000
#   3        0.0        0.00        0.0    0.0    0.0      0  0.000  0.000  0.000
#   4      _____
#   5      _____
#   6      _____
#   7      _____
#   8      _____
#   9      _____
#  10      _____
#  11      _____
#  12      _____
#  13      _____
#  14      _____
#  15      _____
#  16      _____
#
# F1/RUN F2/GEOM F3/QUIT F4/SPAN F5/BENT F6/PSAVE F7/PB F8/PF F9/FILE F10/COL
*****

```


BAG Output

(BAG output is STRUDL input)

```

STRUDL ' ' 'EXAMPLE PROBLEM'
DEBUG ALL & SCAN CONDITIONAL
REORG 5
$
$          BBBBBB      A      GGGGGG
$          BBBBBB      AAA     GGGGGGGG
$          BB   BB   AA AA   GG   GG
$          BB   BB   AA   AA   GG
$          BB   BB   AA   AA   GG
$          BBBBBB   AAAAAA   GG
$          BB   BB   AAAAAA   GG GGGGGG
$          BB   BB   AA   AA   GG GGGGGG
$          BB   BB   AA   AA   GG   GG
$          BBBBBB   AA   AA   GGGGGGGG
$          BBBBBB   AA   AA   GGGGGG
$ *****
$ THE STRUDL CODING FOR THIS ANALYSIS WAS COMPUTER GENERATED USING
$ BAG(PRODUCTION) MOD 21 6.27.90
$ THE FOLLOWING INPUT AND GENERATED STRUDL CODING
$ SHOULD BE VERIFIED TO BE SURE THE GENERATED MODEL IS VALID
$
$ ***** GEOMETRY DATA *****
$
$ CURVE DIR
$ 0=R 1=L RADIUS BC STA TAN AZIMUTH
$          61.9764
$ 0          300. 1025.10 96.8083
$ 1          300. 1275.30 57.9736
$
$ ***** ABUTMENT DATA *****
$
$ ***** RELEASES *****
$ ABUT STA ELEV AZIMUTH LONG F TRANS F VERT F TORSION
$ 1 1105.1 210.1 347.2551 0 0 1 1
$ 4          200.1 12.9779 300 100 1 1
$
$ ***** SPAN DATA *****
$
$ SPAN IZ IY IX AX MODULUS STRUCTURE DEPTH
$ 1 80.0 510.0 6200.0 809.0 61.0 468000. 5.00
$ 2 100.0 510.0 6200.0 809.0 61.0 468000. 5.00
$ 3 80.0 462.4 60951.2 1461.7 122.9 468000. 5.00
$
$ ***** STATIC LOAD DATA *****
$
$ SPAN W WL LF UMASS TEMP
$ 1 1.000 0.100 1.000 0. 35.
$ 2 1.000 0.100 1.000 0.
$ 3 1.000 0.100 1.000 0.
$
$ ***** HINGE DATA *****
$
$ ***** RESTRAINER DATA ***** HINGE DATA *****
$ SPAN HINGE LENGTH AREA MODULUS AREA LENGTH
$ 2 20.0 20.0 3.1 1440000.00 40.0 24.00
$
$ ***** BENT DATA *****
$
$ ELEVATIONS NO C-C COL CAP DATA
$ BT TOP FTG AZIMUTH COL COL MODULUS COL IZ AX CAP LT.
$ 2 207.0 183.0 2.5339 1 0.0 468000. 1 700.0 40.0 32.0
$ 3 203.2 148.5 328.9114 2 24.0 468000. 2 700.0 40.0 32.0

```

```

$
$ ***** BENT RELEASES *****
$
$      BENT          FORCE RELEASES
$      MOM REL TOP    FOR REL FTG      MOM REL FTG
$      2      1 1 1      1      1      1      1      1      1
$      3      1 1 1      1      1      1      1      1      1
$
$ ***** CENTRIFUGAL FORCE DATA ***** COL WIND FORCE *****
$
$ BT  CFH1  CFH2  CFH3  CFP1  CFP2  CFP3      CWFL      CWFT
$ 2   0.    0.    0.    0.    0.    0.      1.000    1.000
$ 3   0.    0.    0.    0.    0.    0.      1.000    1.000
$
$ ***** COLUMN DATA *****
$
$ TYPE  SEG   L   AREA   IZ   IY   IX
$ 1     1    0.0  15.0   11.2  31.2  45.0
$ 2     1    0.0  12.0   9.0   16.0  36.0
$
$ ***** RUN DATA *****
$
$      EXTRA JOINTS
$      SPAN  COL  MAX SPANS  TOTAL JOINTS  MAX JOINTS  SPEC MOD F  PLOT COND
$      3     2     48         28          75      1.0000    1 1
$
$ THE STRUCTURE HAS BEEN ROTATED -21.5180 DEGREES (CLOCKWISE = + )
$ FROM THE INPUT LOCATION TO PERMIT SEISMIC LOADS TO BE APPLIED
$ IN THE GLOBAL X AND Z DIRECTIONS
$
$ *****
$ TYPE SPACE FRAME
$ UNITS FEET KIPS DEGREES LBM
$
$      JOINT NUMBER KEY
$      JOINT  DIGIT 1  DIGITS 2,3  DIGIT 4  DIGIT 5
$      =====
$ SUPERSTRUCTURE  1      SPAN NO.  JOINT NO.  -----
$ HINGE           2      SPAN NO.  JOINT NO.  -----
$ ABUTMENT        3      SUPPORT NO. JOINT NO.  -----
$ COLUMN          4      BENT NO.   COL. NO.  JOINT NO.
$
$ JOINT COORDINATES
$ JOINT      X      Y      Z
$ SUPERSTRUCTURE
$
$ ADDITIONAL COORDINATES TO BE USED FOR THE PHASE2 RETROFIT - TENSION MODEL ONLY
$ COORDINATES MUST BE PROVIDED BY THE ENGINEER
$
$      3001      0.00      0.00      0.00  SUPPORT  $ TENSION MODEL ONLY
$      3051      0.00      0.00      0.00  SUPPORT  $ TENSION MODEL ONLY
$      3011 10000.00  210.10 10000.00  SUPPORT
$      1012 10000.97  210.06 9999.78
$      1013 10020.37  209.32 9996.12
$      1014 10039.98  208.55 9993.75
$      1015 10059.70  207.77 9992.68
$      1021 10079.44  207.00 9992.90
$      2022 10099.38  206.24 9994.45
$      2023 10100.37  206.20 9994.56
$      1024 10119.98  205.46 9996.89
$      1025 10139.60  204.72 9999.23
$      1026 10159.21  203.98 10001.58
$      1031 10178.83  203.24 10003.76
$      1032 10198.55  202.43 10004.81
$      1033 10218.29  201.67 10004.56
$      1034 10237.98  200.90 10003.02
$      1035 10257.52  200.14 10000.17
$      3041 10258.50  200.10 10000.00  SUPPORT
$ COLUMN/S

```

```

40211 10079.44 183.00 9992.90 SUPPORT
40212 10079.44 191.00 9992.90
40213 10079.44 199.00 9992.90
40311 10172.63 148.50 9993.48 SUPPORT
40312 10172.63 166.73 9993.48
40313 10172.63 184.97 9993.48
40314 10172.63 203.20 9993.48
40321 10185.02 148.50 10014.03 SUPPORT
40322 10185.02 166.73 10014.03
40323 10185.02 184.97 10014.03
40324 10185.02 203.20 10014.03
$
$ MEMBER NUMBER KEY
$ MEMBER DIGIT 1 DIGITS 2,3 DIGIT 4 DIGIT 5
$ =====
$ SUPERSTRUCTURE 5 SPAN NO. MEM. NO. -----
$ CAP 6 BENT NO. MEM. NO. -----
$ COLUMN 7 BENT NO. COL. NO. MEM. NO.
$ RESTRAINER 8 SPAN NO. -----
$
MEMBER INCIDENCES
$ MEMBER START END
$ SUPERSTRUCTURE
MEMBER INCIDENCES
5011 3011 1012
5012 1012 1013
5013 1013 1014
5014 1014 1015
5015 1015 1021
5021 1021 2022
5022 2022 2023
5023 2023 1024
5024 1024 1025
5025 1025 1026
5026 1026 1031
5031 1031 1032
5032 1032 1033
5033 1033 1034
5034 1034 1035
5035 1035 3041
TYPE SPACE TRUSS
MEMBER INCIDENCES
802 1021 1024
$ RESTRAINER/S
$
$ ADD RESTRAINERS AT ABUTMENTS FOR USE IN THE PHASE2 RETROFIT-TENSION MODEL ONLY
$
$ 800 3001 1013 $ TENSION MODEL ONLY
$ 803 1034 3051 $ TENSION MODEL ONLY
TYPE SPACE FRAME
MEMBER INCIDENCES
$ COLUMN/S
70211 40211 40212
70212 40212 40213
70213 40213 1021
70311 40311 40312
70321 40321 40322
70312 40312 40313
70322 40322 40323
70313 40313 40314
70323 40323 40324
$ CAP/S
6031 40314 1031
6032 1031 40324
$
$ SUPERSTRUCTURE MEMBERS
$
GROUP '/GP/SUPE' DEFINITION
MEMB 5011 THRU 5015

```

```

MEMB 5021 THRU 5026
MEMB 5031 THRU 5035
END GROUP DEFINITION
$ SUPERSTRUCTURE MEMBER PROPERTIES
MEMBER PROPERTIES PRISMATIC
5011 THRU 5015 AX 61.0 IX 809.0 IY 6200.0 IZ 510.0
5021 THRU 5026 AX 61.0 IX 809.0 IY 6200.0 IZ 510.0
5031 THRU 5035 AX 122.9 IX 1461.7 IY 60951.2 IZ 462.4
$
$ COLUMN MEMBERS
$
GROUP '/GP/COLS' DEFINITION
MEMB 70211 THRU 70213
MEMB 70311 THRU 70313
MEMB 70321 THRU 70323
END GROUP DEFINITION
$ COLUMN MEMBER PROPERTIES
MEMB PROP PRIS AX 15.0 IX 45.0 IY 31.2 IZ 11.2
70211
MEMB PROP PRIS AX 15.0 IX 45.0 IY 31.2 IZ 11.2
70212
MEMB PROP PRIS AX 15.0 IX 45.0 IY 31.2 IZ 11.2
70213
MEMB PROP PRIS AX 12.0 IX 36.0 IY 16.0 IZ 9.0
70311 70321
MEMB PROP PRIS AX 12.0 IX 36.0 IY 16.0 IZ 9.0
70312 70322
MEMB PROP PRIS AX 12.0 IX 36.0 IY 16.0 IZ 9.0
70313 70323
$
$ CAP MEMBERS
$
GROUP '/GP/CAPS' DEFINITION
MEMB 6031 THRU 6032
END GROUP DEFINITION
$ CAP MEMBER PROPERTIES
MEMBER PROPERTIES PRISMATIC
6031 THRU 6032 AX 40.0 IX 700000.0 IY 700000.0 IZ 700.0
$ RESTRAINER MEMBER PROPERTIES
802 AX 0.0439
$
$ AREA OF ADDED RESTRAINERS FOR THE PHASE2 RETROFIT AT THE ABUTMENTS FOR THE
TENSION ONLY MODEL
$
MEMBER PROPERTIES PRISMATIC
$ 800 AX 0. $ TENSION ONLY MODEL ADD RESTRAINER AREA
$ 803 AX 0. $ TENSION ONLY MODEL ADD RESTRAINER AREA
$
CONSTANTS
DENSITY 150.0 ALL
G 172800. ALL
CTE 0.0000065 ALL
$ SUPERSTRUCTURE MODULUS OF ELASTICITY
E 468000. 5011 THRU 5015
E 468000. 5021 THRU 5026
E 468000. 5031 THRU 5035
$ COLUMN/S MODULUS OF ELASTICITY
E 468000. 70211 THRU 70213
E 468000. 70311 THRU 70313
E 468000. 70321 THRU 70323
$ CAP/S MODULUS OF ELASTICITY
E 468000. 6031 THRU 6032
$ RESTRAINER MODULUS OF ELASTICITY
$
$ MODULUS OF ELASTICITY FOR THE ADDED RESTRAINERS FOR THE PHASE2 RETROFIT AT THE
ABUTMENTS FOR THE TENSION ONLY MODEL
$
E 1440000. 802
$

```

```

$ E 1440000. 800 $ TENSION ONLY MODEL
$ E 1440000. 803 $ TENSION ONLY MODEL
$ BETA ANGLES FOR COLUMNS
BETA -2.5339 70211 THRU 70213
BETA 31.0886 70311 THRU 70313
BETA 31.0886 70321 THRU 70323
$ ABUTMENT 1 RELEASES
$
$ MEMBER RELEASE REQUIRED FOR THE TENSION ONLY MODEL
$
MEMBER 5011 REL END FOR X Z
JOINT 3011 RELEASE MOM Y Z -
TH2 -12.7449
$ ABUTMENT 4 RELEASES
$
$ MEMBER RELEASE REQUIRED FOR THE TENSION ONLY MODEL
$
MEMBER 5035 REL STA FOR X $ TENSION ONLY MODEL
$
JOINT 3041 RELEASE MOM Y Z -
TH2 12.9778 KFX 3600. KFZ 1200.
$ HINGE RELEASES
MEMBER 5022 RELEASES END FORCE X MOMENT Y Z
$ COLUMN RELEASES
$ COLUMN MEMBER END JOINT SIZE
MEMBER END JOINT SIZE
MEMBER 70213 END SIZE END 2.50
MEMBER 70313 END SIZE END 2.50
MEMBER 70323 END SIZE END 2.50
$
$ REMOVE MEMBER RELEASES FORCE X AT ABUTMENTS AND HINGES FOR THE COMPRESSION
$ MODEL
$
$ INACTIVATE RESTRAINERS AT HINGES AND ABUTMENTS WHEN USING THE
$ COMPRESSION ONLY MODEL
$
$ INACTIVE JOINT 3001 3051 $ COMPRESSION ONLY MODEL
$ INACTIVE MEMBERS 802 $ COMPRESSION ONLY MODEL
$ INACTIVE MEMBERS 800 $ COMPRESSION ONLY MODEL
$ INACTIVE MEMBERS 803 $ COMPRESSION ONLY MODEL
INERTIA OF JOINTS LUMPED
$ HINGE WEIGHT
INERTIA OF JOINTS ADD
2022 LINEAR ALL 72000.00
2023 LINEAR ALL 72000.00
$ CAP WEIGHT
INERTIA OF JOINTS ADD
1021 LINEAR ALL 192000.00
40314 LINEAR ALL 24000.00
40324 LINEAR ALL 24000.00
DAMPING PERCENTS 5.0 12
SHOCK SPECTRUM LOAD ' Z DIR' 'EXAMPLE PROBLEM '
SPECTRUM TRANSLATION Z 1.000 X 0.000 FILE 'S.6GA51' FACTOR 1.0000
END OF DYNAMIC LOAD
SHOCK SPECTRUM LOAD ' X DIR' 'EXAMPLE PROBLEM '
SPECTRUM TRANSLATION Z 0.000 X 1.000 FILE 'S.6GA51' FACTOR 1.0000
END OF DYNAMIC LOAD
LOAD ' TRANS'
CQC LOAD ' Z DIR'
LOAD 'LONGIT'
CQC LOAD ' X DIR'
LOAD COMB 'CASE 1' 'TRANS PLUS 0.3 TIMES LONGIT CQC'
LOAD COMB 'CASE 2' 'LONGIT PLUS 0.3 TIMES TRANS CQC'
LOAD ' TRANSC'
PRINT STRUCTURAL DATA
DEFINE INERTIA WEIGHT LOAD 'D' WITH FACTORS LINEAR Y -1.
LOADING LIST 'D'
STIFFNESS ANALYSIS REDUCE BAND
OUTPUT DECIMAL 5

```

```

LIST DISPLACEMENTS ALL
UNITS INCHES
PLOT DEVICE PLOTTER PAPER 30
PLOT FORMAT ORIENTATION NON STANDARD 12.8
LABEL JOINTS
TITLE 'EXAMPLE PROBLEM'
TITLE BLOCK
JOB 'EXAMPLE PROBLEM'
FOR ' '
BY ' BAG '
DISPLAY THREE DIMENSIONAL
PLOT FINISH
UNITS FEET
STATISTICS PARAMETERS
INERTIA MATRIX LUMPED
KINEMATIC CONDENSATION INDEPENDENT 72
MODAL ANALYSIS 12
STATISTICS ANALYSIS REDUCE BAND
END STATISTICS
ASSEMBLE FOR DYNAMICS REDUCE BAND
INDEPENDENT DEGREE OF FREEDOM SELECT
CONDENSE DYNAMIC MATRICES
DUMP ORTHOGONALITY
MODAL ANALYSIS HOW 12
LIST DYNAMIC NORMALIZED EIGENVECTORS
LIST DYNAMIC PARTICIPATION FACTORS
$ LIST DYNAMIC NORMALIZED EIGENVECTORS BY JOINT -
$ 3011 1012 1021 2022 2023 1031 1035 3041
SHOCK SPECTRUM ANALYSIS LOAD ' Z DIR' ' X DIR'
GENERATE RESULTS FOR LOADS ' TRANS' 'LONGIT'
COMBINE 'CASE 1' ' TRANS' 1.0 'LONGIT' 0.3
COMBINE 'CASE 2' 'LONGIT' 1.0 ' TRANS' 0.3
LOAD LIST 'CASE 1' 'CASE 2'
OUTPUT DECIMAL 1
$ LIST ELASTIC COL MEM FORCES (A*R*S LOAD)
LIST FORCES MEMBERS -
70211 70213 -
70311 70313 -
70321 70323
$ LIST ELAS. RESTRAINER FORCES (A*R*S LOAD)
LIST FORCES MEMBERS -
802
$
$ LIST FORCES DIST MEMS -
$ 800 $ TENSION ONLY MODEL
$ 803 $ TENSION ONLY MODEL
$ LIST ELAS. GLOBAL DISP FOR MAJOR JOINTS (A*R*S LOAD)
OUTPUT DECIMAL 4
LIST DISPLACEMENTS JOINTS -
3011 1012 1021 2022 2023 1031 1035 3041
OUTPUT DECIMAL 1
$ LIST ELAS MEM FORCES AT ABUTS AND HINGES (A*R*S LOAD)
LIST FORCES MEMBERS -
5011 5022 5035
FINISH
LOAD LIST ' Z DIR' ' X DIR'
OUTPUT DECIMAL 4
LIST DYNAMIC MODAL SHOCK SPECT FORCES MEMS -
70211 70213 -
70311 70313 -
70321 70323
LIST DYNAMIC MODAL SHOCK SPECT DISP JOINTS -
3011 1012 1021 2022 2023 1031 1035 3041
LIST DYNAMIC MODAL SHOCK SPECT FORCES DIST MEMS -
802
$
$ LIST DYNAMIC MODAL SHOCK SPECT FORCES DIST MEMS -
$ 800 $ TENSION ONLY MODEL

```

```

$ 803      $ TENSION ONLY MODEL
$ LIST MODAL MEM FORCES AT ABUTS AND HINGES
LIST DYNAMIC MODAL SHOCK SPECT FORCES MEMS -
5011 5022 5035
FINISH
CHANGES
JOINT 3041 REL MOMENT  Y  Z  -
      KFX 1.0      KFZ 1.0
ADDITIONS
PRINT JOINT RELEASES
$ BRIDGES WITH SHARP RADIUS CURVES(CURVES LESS THAN OR EQUAL
$ TO 1000.0 FEET) MAY GIVE A STATIC CHECK UNBALANCE MESSAGE.
$ IGNORE THIS MESSAGE AS THE RESULTS OF THE STATICS RUN ARE
$ SATISFACTORY.
$
INACTIVE MEMBERS -
802
$ 800      $ TENSION MODEL ONLY
$ 803      $ TENSION MODEL ONLY
LOADING  'W'      'WIND LOAD ON STRUCTURE'
MEMBER LOADS
      5011 THRU 5015 FORCE Z GLOBAL PROJ UNI W 1.000
      5021 THRU 5026 FORCE Z GLOBAL PROJ UNI W 1.000
      5031 THRU 5035 FORCE Z GLOBAL PROJ UNI W 1.000
      5011 THRU 5015 FORCE X          UNI W 0.240
      5021 THRU 5026 FORCE X          UNI W 0.240
      5031 THRU 5035 FORCE X          UNI W 0.240
      70211 THRU 70213 FORCE Z GLOBAL PROJ UNI W 1.000
      70311 THRU 70313 FORCE Z GLOBAL PROJ UNI W 1.000
      70321 THRU 70323 FORCE Z GLOBAL PROJ UNI W 1.000
      70211 THRU 70213 FORCE X GLOBAL PROJ UNI W 1.000
      70311 THRU 70313 FORCE X GLOBAL PROJ UNI W 1.000
      70321 THRU 70323 FORCE X GLOBAL PROJ UNI W 1.000
LOADING  'WL'      'WIND LOAD ON LIVE LOAD'
MEMBER LOADS
      5011 THRU 5015 FORCE Z GLOBAL PROJ UNI W 0.100
      5011 THRU 5015 MOM  X GLOBAL PROJ UNI W 0.850
      5021 THRU 5026 FORCE Z GLOBAL PROJ UNI W 0.100
      5021 THRU 5026 MOM  X GLOBAL PROJ UNI W 0.850
      5031 THRU 5035 FORCE Z GLOBAL PROJ UNI W 0.100
      5031 THRU 5035 MOM  X GLOBAL PROJ UNI W 0.850
      5011 THRU 5015 FORCE X          UNI W 0.040
      5011 THRU 5015 MOM  Z          UNI W 0.340
      5021 THRU 5026 FORCE X          UNI W 0.040
      5021 THRU 5026 MOM  Z          UNI W 0.340
      5031 THRU 5035 FORCE X          UNI W 0.040
      5031 THRU 5035 MOM  Z          UNI W 0.340
LOADING  'LF'      'LONGITUDINAL FORCE'
MEMBER LOADS
      5011 THRU 5015 FORCE X          UNI W 1.000
      5011 THRU 5015 MOM  Z          UNI W 8.500
      5021 THRU 5026 FORCE X          UNI W 1.000
      5021 THRU 5026 MOM  Z          UNI W 8.500
      5031 THRU 5035 FORCE X          UNI W 1.000
      5031 THRU 5035 MOM  Z          UNI W 8.500
$
$ LOADING  'T'      'TEMPERATURE LOAD'
MEMBER TEMPERATURE LOADS
'/GP/SUPE' AXIAL 35.
'/GP/COLS' AXIAL 35.
'/GP/CAPS' AXIAL 35.
$
$ FOR BENT 2
$
$
$ LOADING  'DY - 2'  'TRANS MOMENT DISTRIBUTION'
$ FOR SINGLE COLUMN BENTS USE THE FOLLOWING COMMAND (ADD + 1 OR - 1 TO
$ THE ANSWER)

```



```

MEMBER 70213 END LOAD END MOMENT Y 1.0
$
LOADING 'DX - 2' 'LONGIT MOMENT DISTRIBUTION'
$ FOR SINGLE COLUMN BENTS USE THE FOLLOWING COMMAND (ADD + 1 OR - 1 TO
$ THE ANSWER)
MEMBER 70213 END LOAD END MOMENT Z 1.0
$
$ FOR BENT 3
$
$
LOADING 'DY - 3' 'TRANS MOMENT DISTRIBUTION'
MEMBER 6031 END LOAD START MOMENT Z 1.0
$
LOADING 'DX - 3' 'LONGIT MOMENT DISTRIBUTION'
MEMBER 6031 END LOAD START MOMENT X 1.0
STIFFNESS ANALYSIS REDUCE BAND
$ THE COMMAND ABOVE LISTS ALL RESULTS
OUTPUT DECIMAL 2
$ THE FOLLOWING COMMANDS REFORMAT THE COLUMN FORCES ONLY
OUTPUT BY MEMBER
$ LIST COLUMN MEMBER FORCES
LOADING LIST 'CASE 1' 'CASE 2' 'W' 'WL' 'LF' 'T' 'DY - 2', -
'DX - 2' 'CFH1- 2' 'CFH2- 2' 'CFH3- 2' 'CFP1- 2' 'CFP2- 2', -
'CFP3- 2'
LIST FORCES MEMBER -
70211 70213
LOADING LIST 'CASE 1' 'CASE 2' 'W' 'WL' 'LF' 'T' 'DY - 3', -
'DX - 3' 'CFH1- 3' 'CFH2- 3' 'CFH3- 3' 'CFP1- 3' 'CFP2- 3', -
'CFP3- 3'
LIST FORCES MEMBER -
70311 70313 -
70321 70323
FINISH

```

EXAMPLE 2

PROBLEM STATEMENT

Generate BAG input coding to analyze the two span structure shown in Figure 3 using the given properties with given origin coordinates and no rotation.

Superstructure Properties

| | | |
|-------------------------|----------|----------------------|
| Span length | - span 1 | 120 ft |
| | span 2 | 80 ft |
| Total length | | 200 ft |
| Cross-sectional area | | 61 ft ² |
| Moments of inertia | | |
| Torsion (IX) | | 809 ft ⁴ |
| Normal Bending (IZ) | | 510 ft ⁴ |
| Transverse Bending (IY) | | 6200 ft ⁴ |
| Modulus of Elasticity | | 468000 ksf |

Substructure Properties

Bent 2

| | |
|-------------------|----------|
| Top Elevation | 210.0 ft |
| Bottom Elevation | 190.0 ft |
| Number of columns | 1 |

Column

| | |
|-------------------------|----------------------|
| Cross-sectional area | 15.0 ft ² |
| Moments of inertia | |
| Torsion (IX) | 45.0 ft ⁴ |
| Normal Bending (IZ) | 11.2 ft ⁴ |
| Transverse Bending (IY) | 31.2 ft ⁴ |
| Modulus of Elasticity | 468000.0 ksf |

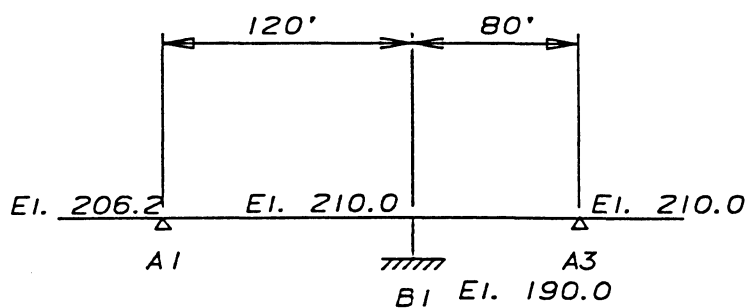
Seismic Loading (CALTRANS)

| | |
|-------------------------------|---------|
| Peak rock acceleration | 0.6 g |
| Depth to "rock-like" material | 0-10 ft |

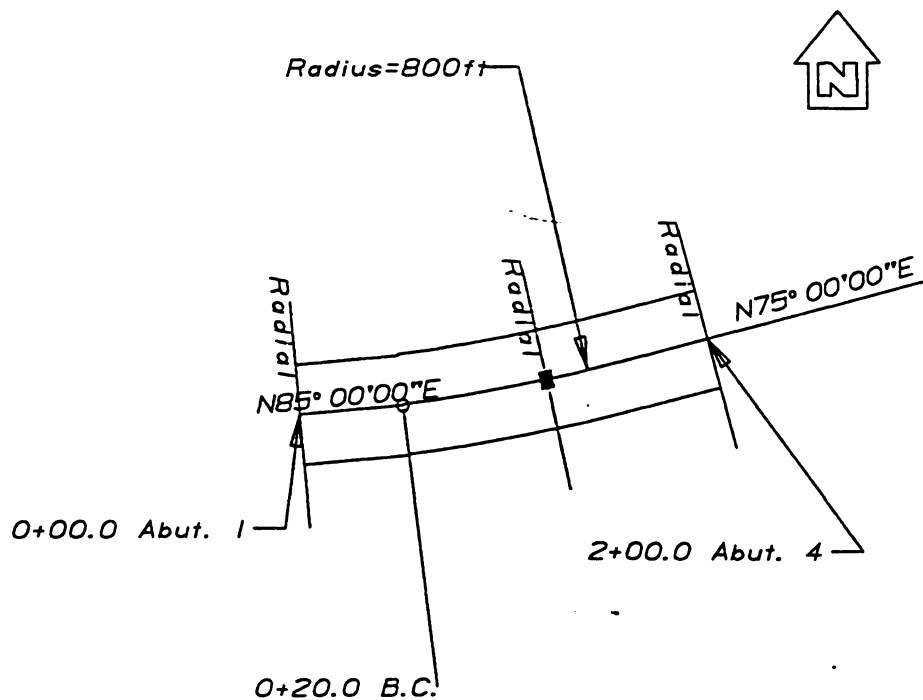
Horizontal Alignment

Curve 1

| | |
|----------------------|-------------|
| Tangent Bearings | |
| BC to PI | N85 00'00"E |
| PI to EC | N75 00'00"E |
| Radius | 300 ft |
| Station BC | 0+20.00 |
| Station Abutment 1 | 0+00.00 |
| Elevation Abutment 1 | 206.20 |
| Elevation Abutment 4 | 210.00 |



LINE DIAGRAM-ELEVATION



PLAN

Figure 3

BAG - - EXAMPLE PROBLEM 2

BAG Panel Input

Input Data File

```

|.....1.....2.....3.....4.....5.....6.....7.....8
1      EXAMPLE 2                      1  996265 101029110
2      1N850000EL  800    2000N750000E
3          0 2062          0 0 2100          0 0
4      11200 5100 62000 8090 610 468 0 0 0 0 0 0 550
4      2 800 0 0 0 0 0 0 0 0 0 0 0 0
5      1 0 0 0 0 0 0 0 0 0 0 0 0
5      2 0 0 0 0 0 0 0 0 0 0 0 0
6      2 2100 1900 1 0 0 0 0 400 3200
7      0 1 0 150 112 312 450
8      2000 0 0 0 0 0 0 0 0
9      6A 0 0 0 0 0 0 0 0
10     1 0 0 0 0 0 0
10     2 0 0 0 0 0
11     2 0 0 0 0 0 0 0

```

```
*****  
*  
*  
*                               BAG                               V 3.01 RELEASE MAY 18, 1988  
*  
* RUN DATA  
*  
* COMMENTS ==> Example 2 _____  
* NOM NJS NJC JMAX NSPX      SF   PLOT ANALYSIS ROTATION REDUCE I/O CONDEN.  
*    - - - - -          -       -           1             -  
*  
* NOM : NO. OF MODES      ( DEFAULT = 12 OR 3 * NO OF SPANS )  
* NJS : NO. OF EXTRA JOINTS ADDED PER SPAN ( DEFAULT = 3 )  
* NJC : NO. OF EXTRA JOINTS ADDED PER COL. ( DEFAULT = 2 )  
* JMAX: MAX. NO. OF JOINTS              ( DEFAULT = 75 )  
* NSPX: MAX. NO. OF SPANS                ( DEFAULT = 48 )  
* PLOT: PLOT LENGTH CODE ( LENGTH = 18 + 6 * PLOT INCHES, 0 FOR NO PLOT)  
* ANALYSIS: 0 = STATIC & DYNAMIC , 1 = DYNAMIC ONLY , 2 = STATIC ONLY  
* ROTATION: 0 = YES                      , 1 = NO  
*-----  
* EARTHQUAKE LOAD DATA  
* PEAK :CURVE:        UP TO 6 ADDITIONAL OPTIONAL DIRECTIONS FOR GIVEN LOAD  
* ROCK : DES.:         :         :         :         :         :  
* ACC. :         :     2         :     3         :     4         :     5         :     6         :     7  
* 1/10 G: A-D :         :         :         :         :         :         :  
* 6          S - - - - - - - - - - - - - - - - - - - - - - - - - - -  
* SPECIAL SITE SPECTRUM _____ (8 CHARACTERS NAME FOR STRUDL) _____  
*  
* F1/RUN F2/GEOM F3/QUIT F4/SPAN F5/BENT F6/PSAVE F7/PB F8/PF F9/FILE F10/COL  
*  
*****
```

```

*****
*                                     *
*                                     *
*                                     *
*          BAG           V 3.01 RELEASE MAY 18, 1988      *
*                                     *
* RUN DATA                                                *
*                                     *
* COMMENTS ==> EXAMPLE 2                                *
* NOM NJS NJC JMAX NSPX   SF    PLOT ANALYSIS ROTATION  *
*                                     1                    *
*                                     *
* NOM : NO. OF MODES      ( DEFAULT = 12 OR 3 * NO OF SPANS ) *
* NJS : NO. OF EXTRA JOINTS ADDED PER SPAN ( DEFAULT = 3 )   *
* NJC : NO. OF EXTRA JOINTS ADDED PER COL. ( DEFAULT = 2 )   *
* JMAX: MAX. NO. OF JOINTS              ( DEFAULT = 75 )     *
* NSPX: MAX. NO. OF SPANS                ( DEFAULT = 48 )     *
* PLOT: PLOT LENGTH CODE ( LENGTH = 18 + 6 * PLOT INCHES, 0 FOR NO PLOT) *
* ANALYSIS: 0 = STATIC & DYNAMIC , 1 = DYNAMIC ONLY , 2 = STATIC ONLY *
* ROTATION: 0 = YES                      , 1 = NO             *
* -----                                                  *
* EARTHQUAKE LOAD DATA                                    *
* PEAK : CURVE:        UP TO 6 ADDITIONAL OPTIONAL DIRECTIONS FOR GIVEN LOAD *
* ROCK : DES.:         :       :       :       :       :       :       *
* ACC. : :             2       :       3       :       4       :       5       :       6       :       7       *
* 1/10 G: A-D :         :       :       :       :       :       :       *
* 6      A - - - - - (8 CHARACTERS NAME FOR STRUDL) - - - - - *
* SPECIAL SITE SPECTRUM                                         *
* F1/RUN F2/GEOM F3/QUIT F4/SPAN F5/BENT F6/PSAVE F7/PB F8/PF F9/FILE F10/COL *
*                                     *
*****

```

```

*****
*
*
*
*          BAG
*
*   GEOMETRY DATA
*   ORIGIN DATA
*       COORDINATES
*           Z             X
*     9962.65_____  10102.91____
*   CURVE DATA
*         CURVE      DIR      RADIUS      BC      BEARING
*         NO.        R/L      FT          FT      PI TO EC
*           1         1      800_____  20_____  n 750000 e
*           2         -              _____  _____  -
*
* -----
*   ABUTMENT DATA
*
*   ABUTMENT STATION :ELEVATION:    BEARING :    RELEASE
*               FT   :   FT   :      :      FORCE
*               :      :      :      :      : MOMENT
*               :      :      : LONG TRANS V :TORSIONAL
*
*   10    0_____  206.24_  _ _ _ _ _  _ _ _ _ _  _ _ _ _ _
*    0    210_____  _ _ _ _ _  _ _ _ _ _  _ _ _ _ _
*
*   DESIGN TEMPERATURE RANGE (F) : 35_ DEGREES
*   (TEMPERATURE COEFFICIENT IS BASED ON CONCRETE. USER SHOULD CHECK STRUDL CODE.)
*
* F1/RUN F2/GEOM F3/QUIT F4/SPAN F5/BENT F6/PSAVE F7/PB F8/PF F9/FILE F10/COL
*
*****

```

```
*****
```

| GEOMETRY DATA | | | | | |
|---------------|----------|-----------------------------|--------------------------|------------------|--|
| ORIGIN DATA | | TANGENT OR BC TO PI BEARING | ROTATION ANGLE (DEGREES) | | |
| Z | X | | | | |
| 9962.65 | 10102.91 | N 850000 E | 0.0000 | | |
| CURVE DATA | | | | | |
| CURVE NO. | DIR R/L | RADIUS FT | BC FT | BEARING PI TO EC | |
| 1 | L | 800 | 20.0 | N 750000 E | |
| 2 | - | - | - | - | |

```
-----
```

| ABUTMENT DATA | | | | | | |
|---------------|---------|------------|-----------|------------------------|-------|-------------|
| ABUTMENT | STATION | ELEVATION: | BEARING : | RELEASE FORCE : MOMENT | | |
| | FT : | FT : | : | LONG | TRANS | V TORSIONAL |
| 10 | 0.0 | 206.2 | - - | 0 | 0 | |
| 10 | | 210.0 | - - | 0 | 0 | |

DESIGN TEMPERATURE RANGE (F) : 0 DEGREES
(TEMPERATURE COEFFICIENT IS BASED ON CONCRETE. USER SHOULD CHECK STRUDL CODE.)

F1/RUN F2/GEOM F3/QUIT F4/SPAN F5/BENT F6/PSAVE F7/PB F8/PF F9/FILE F10/COL

```
*****
```


BAG Panel Input Screen 6 - Input

```

*****
*
*
*
*          BAG
*
*      BENT DATA - 1
*
*      BENT:  ELEVATION :      :NO.  C-C      E : COL. :  -----  CAP -----
*             : TOP    BOTTOM : BEARING :OF    COL      :      :      IZ      AREA  LENG
*             NO.: (FT)   (FT) :         :COL. (FT)   (KSF) : TYPE : (FT**4) (FT**2) (FT)
*             2  210     190      1      1
*
*      -----
*      -----
*      -----
*      -----
*      -----
*      -----
*      -----
*      -----
*      -----
*      -----
*      -----
*      -----
*      -----
*      -----
*      -----
*
*      F1/RUN F2/GEOM F3/QUIT F4/SPAN F5/BENT F6/PSAVE F7/PB F8/PF F9/FILE F10/COL
*
*****

```

BAG Panel Input Screen 6 - Output

```

*****
*
*
*
*          BAG
*
*      BENT DATA - 1
*
*      BENT:  ELEVATION :      :NO.  C-C      E : COL. :  -----  CAP -----
*             : TOP    BOTTOM : BEARING :OF    COL      :      :      IZ      AREA  LENG
*             NO.: (FT)   (FT) :         :COL. (FT)   (KSF) : TYPE : (FT**4) (FT**2) (FT)
*             2  210.0    190.0  -      1    0.0      0    0      0.0  40.0  32.0
*
*      -----
*      -----
*      -----
*      -----
*      -----
*      -----
*      -----
*      -----
*      -----
*      -----
*      -----
*      -----
*      -----
*      -----
*      -----
*
*      F1/RUN F2/GEOM F3/QUIT F4/SPAN F5/BENT F6/PSAVE F7/PB F8/PF F9/FILE F10/COL
*
*****

```


BAG Output

```

STRUDL ' ' 'EXAMPLE 2
DEBUG ALL & SCAN CONDITIONAL
REORG 5
$          BBBB      A      GGGGG
$          BBBB      AAA     GGGGGGG
$          BB   BB   AA AA   GG   GG
$          BB   BB   AA AA   GG
$          BB   BB   AA AA   GG
$          BBBB      AAAAAA   GG
$          BB   BB   AAAAAA   GG GGGGG
$          BB   BB   AA AA   GG GGGGG
$          BB   BB   AA AA   GG GG
$          BBBB      AA AA   GGGGGGG
$          BBBB      AA AA   GGGGG
$ *****
$ THE STRUDL CODING FOR THIS ANALYSIS WAS COMPUTER GENERATED USING
$ BAG(PRODUCTION) MOD 21 6.27.90
$ THE FOLLOWING INPUT AND GENERATED STRUDL CODING
$ SHOULD BE VERIFIED TO BE SURE THE GENERATED MODEL IS VALID
$
$ ***** GEOMETRY DATA *****
$
$ CURVE DIR
$ 0=R 1=L RADIUS BC STA TAN AZIMUTH
$          84.9999
$ 1      800.    20.00 74.9999
$
$ ***** ABUTMENT DATA *****
$
$ ***** RELEASES *****
$ ABUT STA ELEV AZIMUTH LONG F TRANS F VERT F TORSION
$ 10 0.0 206.2 354.9998 0 1 1 1
$ 12 210.0 344.9998 0 1 1 1
$
$ ***** SPAN DATA *****
$
$ SPAN IZ IY IX AX MODULUS STRUCTURE DEPTH
$ 10 120.0 510.0 6200.0 809.0 61.0 468000. 5.50
$ 11 80.0 510.0 6200.0 809.0 61.0 468000. 5.50
$
$ ***** STATIC LOAD DATA *****
$
$ SPAN W WL LF UMASS TEMP
$ 10 1.000 0.100 1.000 0. 35.
$ 11 1.000 0.100 1.000 0.
$
$ ***** BENT DATA *****
$
$ ELEVATIONS NO C-C COL CAP DATA
$ BT TOP FTG AZIMUTH COL COL MODULUS COL IZ AX CAP LT.
$ 11 210.0 190.0 347.8379 1 0.0 468000. 0 0.0 40.0 32.0
$
$ ***** BENT RELEASES *****
$
$ BENT FORCE RELEASES
$ MOM REL TOP FOR REL FTG MOM REL FTG
$ 11 1 1 1 1 1 1 1
$
$ ***** CENTRIFUGAL FORCE DATA ***** COL WIND FORCE *****
$
$ BT CFH1 CFH2 CFH3 CFP1 CFP2 CFP3 CWFL CWFT
$ 11 0. 0. 0. 0. 0. 0. 1.000 1.000
$
$ ***** COLUMN DATA *****

```

```

$
$ TYPE SEG L AREA IZ IV IX
$ 0 1 0.0 15.0 11.2 31.2 45.0
$
$ ***** R U N D A T A *****
$
$ EXTRA JOINTS
$ SPAN COL MAX SPANS TOTAL JOINTS MAX JOINTS SPEC MOD F PLOT COND
$ 3 2 50 14 75 1.0000 0 1
$
$ THE STRUCTURE HAS BEEN ROTATED 0.0000 DEGREES (CLOCKWISE = + )
$ FROM THE INPUT LOCATION TO PERMIT SEISMIC LOADS TO BE APPLIED
$ IN THE GLOBAL X AND Z DIRECTIONS
$
$ *****
$ TYPE SPACE FRAME
$ UNITS FEET KIPS DEGREES LBM
$
$ JOINT NUMBER KEY
$ JOINT DIGIT 1 DIGITS 2,3 DIGIT 4 DIGIT 5
$ =====
$ SUPERSTRUCTURE 1 SPAN NO. JOINT NO. ----
$ HINGE 2 SPAN NO. JOINT NO. ----
$ ABUTMENT 3 SUPPORT NO. JOINT NO. ----
$ COLUMN 4 BENT NO. COL. NO. JOINT NO.
$
$ JOINT COORDINATES
$ JOINT X Y Z
$ SUPERSTRUCTURE
$
$ ADDITIONAL COORDINATES TO BE USED FOR THE PHASE2 RETROFIT - TENSION MODEL ONLY
$ COORDINATES MUST BE PROVIDED BY THE ENGINEER
$
$ 3091 0.00 0.00 0.00 SUPPORT $ TENSION MODEL ONLY
$ 3131 0.00 0.00 0.00 SUPPORT $ TENSION MODEL ONLY
$ 3101 10102.91 206.20 9962.65 SUPPORT
$ 1102 10103.91 206.23 9962.56
$ 1103 10133.53 207.15 9959.89
$ 1104 10163.07 208.10 9956.35
$ 1105 10192.45 209.05 9951.71
$ 1111 10221.64 210.00 9945.99
$ 1112 10240.90 210.00 9941.59
$ 1113 10260.04 210.00 9936.72
$ 1114 10279.11 210.00 9931.60
$ 1115 10298.19 210.00 9926.49
$ 3121 10299.15 210.00 9926.23 SUPPORT
$ COLUMN/S
$ 41111 10221.64 190.00 9945.99 SUPPORT
$ 41112 10221.64 196.67 9945.99
$ 41113 10221.64 203.33 9945.99
$
$ MEMBER NUMBER KEY
$ MEMBER DIGIT 1 DIGITS 2,3 DIGIT 4 DIGIT 5
$ =====
$ SUPERSTRUCTURE 5 SPAN NO. MEM. NO. ----
$ CAP 6 BENT NO. MEM. NO. ----
$ COLUMN 7 BENT NO. COL. NO. MEM. NO.
$ RESTRAINER 8 SPAN NO. ----
$
$ MEMBER INCIDENCES
$ MEMBER START END
$ SUPERSTRUCTURE
$ MEMBER INCIDENCES
$ 5101 3101 1102
$ 5102 1102 1103
$ 5103 1103 1104
$ 5104 1104 1105
$ 5105 1105 1111
$ 5111 1111 1112

```

```

      5112    1112    1113
      5113    1113    1114
      5114    1114    1115
      5115    1115    3121
TYPE SPACE TRUSS
MEMBER INCIDENCES
$
$ ADD RESTRAINERS AT ABUTMENTS FOR USE IN THE PHASE2 RETROFIT-TENSION MODEL ONLY
$
$      809      3091      1103      $ TENSION MODEL ONLY
$      811      1114      3131      $ TENSION MODEL ONLY
TYPE SPACE FRAME
MEMBER INCIDENCES
$      COLUMN/S
      71111    41111    41112
      71112    41112    41113
      71113    41113    1111
$
$ SUPERSTRUCTURE MEMBERS
$
GROUP      '/GP/SUPE'      DEFINITION
MEMB 5101 THRU 5105
MEMB 5111 THRU 5115
END GROUP DEFINITION
$ SUPERSTRUCTURE MEMBER PROPERTIES
MEMBER PROPERTIES PRISMATIC
5101 THRU 5105 AX 61.0 IX 809.0 IY 6200.0 IZ 510.0
5111 THRU 5115 AX 61.0 IX 809.0 IY 6200.0 IZ 510.0
$
$ COLUMN MEMBERS
$
GROUP      '/GP/COLS'      DEFINITION
MEMB 71111 THRU 71113
END GROUP DEFINITION
$ COLUMN MEMBER PROPERTIES
MEMB PROP PRIS AX 15.0 IX 45.0 IY 31.2 IZ 11.2
71111
MEMB PROP PRIS AX 15.0 IX 45.0 IY 31.2 IZ 11.2
71112
MEMB PROP PRIS AX 15.0 IX 45.0 IY 31.2 IZ 11.2
71113
$
$ CAP MEMBERS
$
GROUP      '/GP/CAPS'      DEFINITION
END GROUP DEFINITION
$ CAP MEMBER PROPERTIES
MEMBER PROPERTIES PRISMATIC
$ RESTRAINER MEMBER PROPERTIES
$
$ AREA OF ADDED RESTRAINERS FOR THE PHASE2 RETROFIT AT THE ABUTMENTS FOR THE
$ TENSION ONLY MODEL
$
MEMBER PROPERTIES PRISMATIC
$      809      AX      0.      $ TENSION ONLY MODEL ADD RESTRAINER AREA
$      811      AX      0.      $ TENSION ONLY MODEL ADD RESTRAINER AREA
$
CONSTANTS
DENSITY 150.0 ALL
G 172800. ALL
CTE 0.0000065 ALL
$ SUPERSTRUCTURE MODULUS OF ELASTICITY
E 468000. 5101 THRU 5105
E 468000. 5111 THRU 5115
$ COLUMN/S MODULUS OF ELASTICITY
E 468000. 71111 THRU 71113
$ RESTRAINER MODULUS OF ELASTICITY
$
$ MODULUS OF ELASTICITY FOR THE ADDED RESTRAINERS FOR THE PHASE2 RETROFIT AT THE

```

```

$ ABUTMENTS FOR THE TENSION ONLY MODEL
$
$
$ E 1440000. 809 $ TENSION ONLY MODEL
$ E 1440000. 811 $ TENSION ONLY MODEL
$ BETA ANGLES FOR COLUMNS
BETA 12.1621 71111 THRU 71113
$ ABUTMENT 10 RELEASES
$
$ MEMBER RELEASE REQUIRED FOR THE TENSION ONLY MODEL
$
MEMBER 5101 REL END FOR X
JOINT 3101 RELEASE MOM Y Z -
TH2 -5.0002
$ ABUTMENT 12 RELEASES
$
$ MEMBER RELEASE REQUIRED FOR THE TENSION ONLY MODEL
$
MEMBER 5115 REL STA FOR X
JOINT 3121 RELEASE MOM Y Z -
TH2 -15.0002
$ COLUMN RELEASES
$ COLUMN MEMBER END JOINT SIZE
MEMBER END JOINT SIZE
MEMBER 71113 END SIZE END 2.75
$
$ REMOVE MEMBER RELEASES FORCE X AT ABUTMENTS AND HINGES FOR THE COMPRESSION
$ MODEL
$
$ INACTIVATE RESTRAINERS AT HINGES AND ABUTMENTS WHEN USING THE
$ COMPRESSION ONLYMODEL
$
$ INACTIVE JOINT 3091 3131 $ COMPRESSION ONLY MODEL
$ INACTIVE MEMBERS 809 $ COMPRESSION ONLY MODEL
$ INACTIVE MEMBERS 811 $ COMPRESSION ONLY MODEL
INERTIA OF JOINTS LUMPED
$ HINGE WEIGHT
INERTIA OF JOINTS ADD
$ CAP WEIGHT
INERTIA OF JOINTS ADD
1111 LINEAR ALL 192000.00
DAMPING PERCENTS 5.0 12
SHOCK SPECTRUM LOAD ' Z DIR' 'EXAMPLE 2 '
SPECTRUM TRANSLATION Z 1.000 X 0.000 FILE 'S.6GA51' FACTOR 1.0000
END OF DYNAMIC LOAD
SHOCK SPECTRUM LOAD ' X DIR' 'EXAMPLE 2 '
SPECTRUM TRANSLATION Z 0.000 X 1.000 FILE 'S.6GA51' FACTOR 1.0000
END OF DYNAMIC LOAD
LOAD ' TRANS'
CQC LOAD ' Z DIR'
LOAD 'LONGIT'
CQC LOAD ' X DIR'
LOAD COMB 'CASE 1' 'TRANS PLUS 0.3 TIMES LONGIT CQC'
LOAD COMB 'CASE 2' 'LONGIT PLUS 0.3 TIMES TRANS CQC'
PRINT STRUCTURAL DATA
DEFINE INERTIA WEIGHT LOAD 'D' WITH FACTORS LINEAR Y -1.
LOADING LIST 'D'
STIFFNESS ANALYSIS REDUCE BAND
OUTPUT DECIMAL 5
LIST DISPLACEMENTS ALL
STATISTICS PARAMETERS
INERTIA MATRIX LUMPED
KINEMATIC CONDENSATION INDEPENDENT 36
MODAL ANALYSIS 12
STATISTICS ANALYSIS REDUCE BAND
END STATISTICS
ASSEMBLE FOR DYNAMICS REDUCE BAND
INDEPENDENT DEGREE OF FREEDOM SELECT
CONDENSE DYNAMIC MATRICES

```

```

DUMP ORTHOGONALITY
MODAL ANALYSIS HOW 12
LIST DYNAMIC NORMALIZED EIGENVECTORS
LIST DYNAMIC PARTICIPATION FACTORS
$ LIST DYNAMIC NORMALIZED EIGENVECTORS BY JOINT -
$ 3101 1102 1111 1115 3121
SHOCK SPECTRUM ANALYSIS LOAD 'Z DIR' 'X DIR'
GENERATE RESULTS FOR LOADS 'TRANS' 'LONGIT' 'TRANSC' 'LONGITC'
COMBINE 'CASE 1' 'TRANS' 1.0 'LONGIT' 0.3
COMBINE 'CASE 2' 'LONGIT' 1.0 'TRANS' 0.3
COMBINE 'CASE 3' 'TRANSC' 1.0 'LONGITC' 0.3
COMBINE 'CASE 4' 'LONGITC' 1.0 'TRANSC' 0.3
LOAD LIST 'CASE 1' 'CASE 2' 'CASE 3' 'CASE 4'
OUTPUT DECIMAL 1
$ LIST ELASTIC COL MEM FORCES (A*R*S LOAD)
LIST FORCES MEMBERS -
71111 71113
$
$ LIST FORCES DIST MEMS -
$ 809 $ TENSION ONLY MODEL
$ 811 $ TENSION ONLY MODEL
$ LIST ELAS. GLOBAL DISP FOR MAJOR JOINTS (A*R*S LOAD)
OUTPUT DECIMAL 4
LIST DISPLACEMENTS JOINTS -
3101 1102 1111 1115 3121
OUTPUT DECIMAL 1
$ LIST ELAS MEM FORCES AT ABUTS AND HINGES (A*R*S LOAD)
LIST FORCES MEMBERS -
5101 5115
FINISH
LOAD LIST 'Z DIR' 'X DIR'
OUTPUT DECIMAL 4
LIST DYNAMIC MODAL SHOCK SPECT FORCES MEMS -
71111 71113
LIST DYNAMIC MODAL SHOCK SPECT DISP JOINTS -
3101 1102 1111 1115 3121
$
$ LIST DYNAMIC MODAL SHOCK SPECT FORCES DIST MEMS -
$ 809 $ TENSION ONLY MODEL
$ 811 $ TENSION ONLY MODEL
$ LIST MODAL MEM FORCES AT ABUTS AND HINGES
LIST DYNAMIC MODAL SHOCK SPECT FORCES MEMS -
5101 5115
FINISH
CHANGES
ADDITIONS
PRINT JOINT RELEASES
$ BRIDGES WITH SHARP RADIUS CURVES(CURVES LESS THAN OR EQUAL
$ TO 1000.0 FEET) MAY GIVE A STATIC CHECK UNBALANCE MESSAGE.
$ IGNORE THIS MESSAGE AS THE RESULTS OF THE STATICS RUN ARE
$ SATISFACTORY.
$
$
$ NOTE: DEAD LOAD IS TO BE USED FOR SINGLE COLUMN BENTS ONLY!
$
DELETION
LOADING 'D'
ADDITION
DEFINE INERTIA WEIGHT LOAD 'D' WITH FACTORS LINEAR Y -1.
LOADING 'W' 'WIND LOAD ON STRUCTURE'
MEMBER LOADS
5101 THRU 5105 FORCE Z GLOBAL PROJ UNI W 1.000
5111 THRU 5115 FORCE Z GLOBAL PROJ UNI W 1.000
5101 THRU 5105 FORCE X UNI W 0.240
5111 THRU 5115 FORCE X UNI W 0.240
71111 THRU 71113 FORCE Z GLOBAL PROJ UNI W 1.000
71111 THRU 71113 FORCE X GLOBAL PROJ UNI W 1.000
LOADING 'WL' 'WIND LOAD ON LIVE LOAD'
MEMBER LOADS

```

| | | | | | | | | | |
|--------------|------|------|------|---------------------|---|-------------|-----|---|-------|
| | 5101 | THRU | 5105 | FORCE | Z | GLOBAL PROJ | UNI | W | 0.100 |
| | 5101 | THRU | 5105 | MOM | X | GLOBAL PROJ | UNI | W | 0.875 |
| | 5111 | THRU | 5115 | FORCE | Z | GLOBAL PROJ | UNI | W | 0.100 |
| | 5111 | THRU | 5115 | MOM | X | GLOBAL PROJ | UNI | W | 0.875 |
| | 5101 | THRU | 5105 | FORCE | X | | UNI | W | 0.040 |
| | 5101 | THRU | 5105 | MOM | Z | | UNI | W | 0.350 |
| | 5111 | THRU | 5115 | FORCE | X | | UNI | W | 0.040 |
| | 5111 | THRU | 5115 | MOM | Z | | UNI | W | 0.350 |
| LOADING | 'LF' | | | 'LONGITUDINAL FORCE | | | | | |
| MEMBER LOADS | | | | | | | | | |
| | 5101 | THRU | 5105 | FORCE | X | | UNI | W | 1.000 |
| | 5101 | THRU | 5105 | MOM | Z | | UNI | W | 8.750 |
| | 5111 | THRU | 5115 | FORCE | X | | UNI | W | 1.000 |
| | 5111 | THRU | 5115 | MOM | Z | | UNI | W | 8.750 |

\$

LOADING 'T' 'TEMPERATURE LOAD'

MEMBER TEMPERATURE LOADS

'/GP/SUPE' AXIAL 35.

'/GP/COLS' AXIAL 35.

\$

\$ FOR BENT 11

\$

\$

LOADING 'DY - 11' 'TRANS MOMENT DISTRIBUTION'

\$ FOR SINGLE COLUMN BENTS USE THE FOLLOWING COMMAND (ADD + 1 OR - 1 TO

\$ THE ANSWER)

MEMBER 71113 END LOAD END MOMENT Y 1.0

\$

LOADING 'DX - 11' 'LONGIT MOMENT DISTRIBUTION'

\$ FOR SINGLE COLUMN BENTS USE THE FOLLOWING COMMAND (ADD + 1 OR - 1 TO

\$ THE ANSWER)

MEMBER 71113 END LOAD END MOMENT Z 1.0

STIFFNESS ANALYSIS REDUCE BAND

\$ THE COMMAND ABOVE LISTS ALL RESULTS

OUTPUT DECIMAL 2

\$ THE FOLLOWING COMMANDS REFORMAT THE COLUMN FORCES ONLY

OUTPUT BY MEMBER

\$ LIST COLUMN MEMBER FORCES

LOADING LIST 'CASE 1' 'CASE 2' 'D' 'W' 'WL' 'LF' 'T' 'DY - 11', -

'DX - 11' 'CFH1- 11' 'CFH2- 11' 'CFH3- 11' 'CFP1- 11' 'CFP2- 11', -

'CFP3- 11'

LIST FORCES MEMBER -

71111 71113

FINISH

EXAMPLE 3**PROBLEM STATEMENT**

Basically, there are two methods to model the 'Y' or forked shape structures.

The first method is to model the main portion of the structure in BAG and let it rotate. Then you need to calculate the new origin coordinates for the rest of the model so that you can use it for the second and later BAG model. Describe the remaining portion of structure with the new origin and give the rotation angle from the main portion. After all models are generate, merge them all together.

The second method is to model the structure without rotation. In this case, no modification of the origin is needed. After all models are generated, merge them all together. Now you need to modify the earthquake force direction to the primary direction of the structure.

The first method is available only through the BAG program whereas the second method can be done from either STRUBAG or BAG. Before the BAG program is available, people often merge models through a different approach and we are going to show this in Example 3. Actually, Example 1 and Example 2 are one 'Y' shape structure. We have generated the main portion of the structure in Example 1, then we calculate the new origin coordinates and new bearing for the remained structure. With all new data, we generate the Example 2 without rotation. From the example 1 and example 2, now we can combine this two STRUDL model together. The example 2 bridge is going to tie into example bridge at the hinge and a new member has to be added into the final coding.

Note: It is necessary to add members to connect different models. A common practice for these members is to make the end joint size very big so that these members act like rigid body. Whenever the end joint size is used the actual member length should be longer than 0.01 feet. Otherwise will cause a STRUDL error.

STRUDL FILE AFTER MERGE

```

STRUDL ' ' 'EXAMPLE PROBLEM 1 & 2 '
DEBUG ALL & SCAN CONDITIONAL
REORG 5

$          BBBB      A      GGGGGG
$          BBBB      AAA     GGGGGGGG
$          BB  BB  AA  AA  GG  GG
$          BB  BB  AA  AA  GG
$          BB  BB  AA  AA  GG
$          BBBB      AAAAAA  GG
$          BB  BB  AAAAAA  GG  GGGGGG
$          BB  BB  AA  AA  GG  GGGGGG
$          BB  BB  AA  AA  GG  GG
$          BBBB      AA  AA  GGGGGGGG
$          BBBB      AA  AA  GGGGGG
$ *****
$ THE STRUDL CODING FOR THIS ANALYSIS WAS COMPUTER GENERATED USING
$ BAG(PRODUCTION) MOD 21 6.27.90
$ THE FOLLOWING INPUT AND GENERATED STRUDL CODING
$ SHOULD BE VERIFIED TO BE SURE THE GENERATED MODEL IS VALID
$
$ ***** GEOMETRY DATA *****
$
$ CURVE DIR
$ 0=R 1=L  RADIUS  BC STA  TAN AZIMUTH
$          61.9764
$ 0          300.  1025.10  96.8083
$ 1          300.  1275.30  57.9736
$
$ ***** ABUTMENT DATA *****
$
$ ***** RELEASES *****
$ ABUT  STA  ELEV  AZIMUTH  LONG F  TRANS F  VERT F  TORSION
$ 1  1105.1  210.1  347.2551  0  0  1  1
$ 4          200.1  12.9779  300  100  1  1
$
$ ***** SPAN DATA *****
$
$ SPAN  IZ  IY  IX  AX  MODULUS  STRUCTURE DEPTH
$ 1  80.0  510.0  6200.0  809.0  61.0  468000.  5.00
$ 2  100.0  510.0  6200.0  809.0  61.0  468000.  5.00
$ 3  80.0  462.4  60951.2  1461.7  122.9  468000.  5.00
$
$ ***** STATIC LOAD DATA *****
$
$ SPAN  W  WL  LF  UMASS  TEMP
$ 1  1.000  0.100  1.000  0.  35.
$ 2  1.000  0.100  1.000  0.
$ 3  1.000  0.100  1.000  0.
$
$ ***** HINGE DATA *****
$
$ ***** RESTRAINER DATA ***** HINGE DATA *****
$ SPAN HINGE  LENGTH  AREA  MODULUS  AREA  LENGTH
$ 2  20.0  20.0  3.1  1440000.00  40.0  24.00
$
$ ***** BENT DATA *****
$
$ ELEVATIONS  NO C-C COL  CAP DATA
$ BT  TOP  FTG  AZIMUTH COL COL MODULUS COL  IZ  AX  CAP LT.
$ 2  207.0  183.0  2.5339 1  0.0  468000.  1  700.0  40.0  32.0
$ 3  203.2  148.5  328.9114 2  24.0  468000.  2  700.0  40.0  32.0
$
$ ***** BENT RELEASES *****
$

```

```

$ BENT                FORCE RELEASES
$      MOM REL TOP      FOR REL FTG      MOM REL FTG
$      2      1 1 1      1      1      1      1      1      1
$      3      1 1 1      1      1      1      1      1      1
$
$ ***** CENTRIFUGAL      FORCE      DATA *****      COL WIND      FORCE *****
$
$ BT  CFH1  CFH2  CFH3  CFP1  CFP2  CFP3      CWFL      CWFT
$ 2   0.    0.    0.    0.    0.    0.      1.000      1.000
$ 3   0.    0.    0.    0.    0.    0.      1.000      1.000
$
$ ***** C O L U M N      D A T A *****
$
$ TYPE  SEG   L   AREA      IZ      IY      IX
$ 1     1     0.0  15.0     11.2    31.2    45.0
$ 2     1     0.0  12.0      9.0    16.0    36.0
$
$ ***** R U N      D A T A *****
$
$      EXTRA JOINTS
$      SPAN  COL  MAX SPANS  TOTAL JOINTS  MAX JOINTS  SPEC MOD F  PLOT COND
$      3     2     48       28         75      1.0000     1 1
$
$ THE STRUCTURE HAS BEEN ROTATED -21.5180 DEGREES (CLOCKWISE = + )
$ FROM THE INPUT LOCATION TO PERMIT SEISMIC LOADS TO BE APPLIED
$ IN THE GLOBAL X AND Z DIRECTIONS
$
$ *****
$ TYPE SPACE FRAME
$ UNITS FEET KIPS DEGREES LBM
$
$      JOINT NUMBER KEY
$      JOINT      DIGIT 1  DIGITS 2,3  DIGIT 4  DIGIT 5
$      =====  =====  =====  =====  =====
$ SUPERSTRUCTURE  1      SPAN NO.  JOINT NO.  -----
$ HINGE           2      SPAN NO.  JOINT NO.  -----
$ ABUTMENT        3      SUPPORT NO. JOINT NO.  -----
$ COLUMN          4      BENT NO.   COL. NO.  JOINT NO.
$
$ JOINT COORDINATES
$ JOINT      X      Y      Z
$ SUPERSTRUCTURE
$ 3011  10000.00  210.10  10000.00  SUPPORT
$ 1012  10000.97  210.06  9999.78
$ 1013  10020.37  209.32  9996.12
$ 1014  10039.98  208.55  9993.75
$ 1015  10059.70  207.77  9992.68
$ 1021  10079.44  207.00  9992.90
$ 2022  10099.38  206.24  9994.45
$ 2023  10100.37  206.20  9994.56
$ 1024  10119.98  205.46  9996.89
$ 1025  10139.60  204.72  9999.23
$ 1026  10159.21  203.98  10001.58
$ 1031  10178.83  203.24  10003.76
$ 1032  10198.55  202.43  10004.81
$ 1033  10218.29  201.67  10004.56
$ 1034  10237.98  200.90  10003.02
$ 1035  10257.52  200.14  10000.17
$ 3041  10258.50  200.10  10000.00  SUPPORT
$
$ COLUMN/S
$ 40211  10079.44  183.00  9992.90  SUPPORT
$ 40212  10079.44  191.00  9992.90
$ 40213  10079.44  199.00  9992.90
$ 40311  10172.63  148.50  9993.48  SUPPORT
$ 40312  10172.63  166.73  9993.48
$ 40313  10172.63  184.97  9993.48
$ 40314  10172.63  203.20  9993.48
$ 40321  10185.02  148.50  10014.03  SUPPORT
$ 40322  10185.02  166.73  10014.03

```

```

40323 10185.02 184.97 10014.03
40324 10185.02 203.20 10014.03
$ |-----|
$ |          EXAMPLE 2          |
$ V                               V
$   SUPERSTRUCTURE
3101 10102.91 206.20 9962.65 $ <-- MODIFIED
1102 10103.91 206.23 9962.56
1103 10133.53 207.15 9959.89
1104 10163.07 208.10 9956.35
1105 10192.45 209.05 9951.71
1111 10221.64 210.00 9945.99
1112 10240.90 210.00 9941.59
1113 10260.04 210.00 9936.72
1114 10279.11 210.00 9931.60
1115 10298.19 210.00 9926.49
3121 10299.15 210.00 9926.23 SUPPORT
$   COLUMN/S
41111 10221.64 190.00 9945.99 SUPPORT
41112 10221.64 196.67 9945.99
41113 10221.64 203.33 9945.99
$ A                               A
$ |          EXAMPLE 2          |
$ |-----|
$
$   MEMBER NUMBER KEY
$   MEMBER      DIGIT 1  DIGITS 2,3  DIGIT 4  DIGIT 5
$ =====
$ SUPERSTRUCTURE    5      SPAN NO.  MEM. NO.  -----
$ CAP               6      BENT NO.  MEM. NO.  -----
$ COLUMN           7      BENT NO.  COL. NO.  MEM. NO.
$ RESTRAINER       8      SPAN NO.  -----  -----
$
$   MEMBER INCIDENCES
$ MEMBER  START  END
$   SUPERSTRUCTURE
5011 3011 1012
5012 1012 1013
5013 1013 1014
5014 1014 1015
5015 1015 1021
5021 1021 2022
5022 2022 2023
5023 2023 1024
5024 1024 1025
5025 1025 1026
5026 1026 1031
5031 1031 1032
5032 1032 1033
5033 1033 1034
5034 1034 1035
5035 1035 3041
$   TYPE SPACE TRUSS
$   RESTRAINER/S
$   MEMBER INCIDENCES
802 1021 1024
$   TYPE SPACE FRAME
$   MEMBER INCIDENCES
$   COLUMN/S
70211 40211 40212
70212 40212 40213
70213 40213 1021
70311 40311 40312
70321 40321 40322
70312 40312 40313
70322 40322 40323
70313 40313 40314
70323 40323 40324
$   CAP/S

```

```

      6031  40314  1031
      6032  1031  40324
$ HINGE/S
$ 6101  2022  3101 $ <----- ADD
$ |-----|
$ | EXAMPLE 2 |
$ V | V
$ SUPERSTRUCTURE
$ 5101  3101  1102
$ 5102  1102  1103
$ 5103  1103  1104
$ 5104  1104  1105
$ 5105  1105  1111
$ 5111  1111  1112
$ 5112  1112  1113
$ 5113  1113  1114
$ 5114  1114  1115
$ 5115  1115  3121
$ COLUMN/S
$ 71111  41111  41112
$ 71112  41112  41113
$ 71113  41113  1111
$ A | A
$ | EXAMPLE 2 |
$ |-----|
$
$ SUPERSTRUCTURE MEMBERS
$
$ GROUP '/GP/SUPE' DEFINITION
$ MEMB 5011 THRU 5015
$ MEMB 5021 THRU 5026
$ MEMB 5031 THRU 5035
$ |-----|
$ | EXAMPLE 2 |
$ V | V
$ MEMB 5101 THRU 5105
$ MEMB 5111 THRU 5115
$ A | A
$ | EXAMPLE 2 |
$ |-----|
$
$ END GROUP DEFINITION
$ SUPERSTRUCTURE MEMBER PROPERTIES
$ MEMBER PROPERTIES PRISMATIC
$ 5011 THRU 5015 AX 61.0 IX 809.0 IY 6200.0 IZ 510.0
$ 5021 THRU 5026 AX 61.0 IX 809.0 IY 6200.0 IZ 510.0
$ 5031 THRU 5035 AX 122.9 IX 1461.7 IY 60951.2 IZ 462.4
$ |-----|
$ | EXAMPLE 2 |
$ V | V
$ 5101 THRU 5105 AX 61.0 IX 809.0 IY 6200.0 IZ 510.0
$ 5111 THRU 5115 AX 61.0 IX 809.0 IY 6200.0 IZ 510.0
$ A | A
$ | EXAMPLE 2 |
$ |-----|
$
$ COLUMN MEMBERS
$
$ GROUP '/GP/COLS' DEFINITION
$ MEMB 70211 THRU 70213
$ MEMB 70311 THRU 70313
$ MEMB 70321 THRU 70323
$ |-----|
$ | EXAMPLE 2 |
$ V | V
$ MEMB 71111 THRU 71113
$ A | A
$ | EXAMPLE 2 |
$ |-----|
$

```

```

END      GROUP DEFINITION
$ COLUMN MEMBER PROPERTIES
MEMB PROP PRIS AX   15.0 IX      45.0 IY      31.2 IZ      11.2
70211
MEMB PROP PRIS AX   15.0 IX      45.0 IY      31.2 IZ      11.2
70212
MEMB PROP PRIS AX   15.0 IX      45.0 IY      31.2 IZ      11.2
70213
MEMB PROP PRIS AX   12.0 IX      36.0 IY      16.0 IZ       9.0
70311 70321
MEMB PROP PRIS AX   12.0 IX      36.0 IY      16.0 IZ       9.0
70312 70322
MEMB PROP PRIS AX   12.0 IX      36.0 IY      16.0 IZ       9.0
70313 70323
$ |-----|
$ |          EXAMPLE 2          |
$ V                               V
MEMB PROP PRIS AX   15.0 IX      45.0 IY      31.2 IZ      11.2
71111
MEMB PROP PRIS AX   15.0 IX      45.0 IY      31.2 IZ      11.2
71112
MEMB PROP PRIS AX   15.0 IX      45.0 IY      31.2 IZ      11.2
71113
$ A                               A
$ |          EXAMPLE 2          |
$ |-----|
$
$ CAP MEMBERS
$
GROUP '/GP/CAPS'      DEFINITION
MEMB 6031 THRU 6032
MEMB 6101              $ <----- ADD
END GROUP DEFINITION
$ CAP MEMBER PROPERTIES
MEMBER PROPERTIES PRISMATIC
6031 THRU 6032 AX   40.0 IX      700000.0 IY      700000.0 IZ      700.0
6101              AX   40.0 IX      700000.0 IY      700000.0 IZ      700.0 $ ADD
$ RESTRAINER MEMBER PROPERTIES
MEMBER PROPERTIES PRISMATIC
802 AX   0.0439
CONSTANTS
DENSITY 150.0 ALL
G 172800. ALL
CTE 0.0000065 ALL
$ SUPERSTRUCTURE MODULUS OF ELASTICITY
E 468000. 5011 THRU 5015
E 468000. 5021 THRU 5026
E 468000. 5031 THRU 5035
$ COLUMN/S MODULUS OF ELASTICITY
E 468000. 70211 THRU 70213
E 468000. 70311 THRU 70313
E 468000. 70321 THRU 70323
$ CAP/S MODULUS OF ELASTICITY
E 468000. 6031 THRU 6032
E 468000. 6101          $ <----- ADD
$ RESTRAINER MODULUS OF ELASTICITY
E 1440000. 802
$ BETA ANGLES FOR COLUMNS
BETA -2.5339 70211 THRU 70213
BETA 31.0886 70311 THRU 70313
BETA 31.0886 70321 THRU 70323
$ |-----|
$ |          EXAMPLE 2          |
$ V                               V
$ SUPERSTRUCTURE MODULUS OF ELASTICITY
E 468000. 5101 THRU 5105
E 468000. 5111 THRU 5115
$ COLUMN/S MODULUS OF ELASTICITY

```

```

E 468000. 71111 THRU 71113
$ BETA ANGLES FOR COLUMNS
  BETA 12.1621 71111 THRU 71113
$ A
$ |
$ |-----|
$ ABUTMENT 1 RELEASES
  MEMBER 5011 REL END FOR X Z
  JOINT 3011 RELEASE          MOM    Y Z -
  TH2 -12.7449
$ ABUTMENT 4 RELEASES
  JOINT 3041 RELEASE          MOM    Y Z -
  TH2 12.9778 KFX          3600. KFZ    1200.
$ HINGE RELEASES
  MEMBER 5022 RELEASES END FORCE X MOMENT Y Z
$ |-----|
$ |
$ |-----|
$ V
$ ABUTMENT 10 RELEASES
  MEMBER 5101 REL END FOR X
  JOINT 3101 RELEASE          MOM    Y Z -
  TH2 5.5129
$ ABUTMENT 12 RELEASES
  MEMBER 5115 REL STA FOR X
  JOINT 3121 RELEASE          MOM    Y Z -
  TH2 -4.4871
$ A
$ |
$ |-----|
$ MEMBER 6101 JOINT END SIZE START 31.995      $ <----- ADD
  INERTIA OF JOINTS LUMPED
$ HINGE WEIGHT
  INERTIA OF JOINTS ADD
    2022  LINEAR ALL          72000.00
    2023  LINEAR ALL          72000.00
    3101  LINEAR ALL          72000.00      $ <----- ADD
    1102  LINEAR ALL          72000.00      $ <----- ADD
$ CAP WEIGHT
  INERTIA OF JOINTS ADD
    1021  LINEAR ALL          192000.00
    40314 LINEAR ALL          24000.00
    40324 LINEAR ALL          24000.00
$ |-----|
$ |
$ |-----|
$ V
$ CAP WEIGHT
  INERTIA OF JOINTS ADD
    1111  LINEAR ALL          192000.00
$ A
$ |
$ |-----|
$ DAMPING PERCENTS 5.0 15      $ <----- MODIFIED
  SHOCK SPECTRUM LOAD ' Z DIR' 'EXAMPLE PROBLEM
  SPECTRUM TRANSLATION Z FILE 'S.6GA51' FACTOR 1.0000
  END OF DYNAMIC LOAD
  SHOCK SPECTRUM LOAD ' X DIR' 'EXAMPLE PROBLEM
  SPECTRUM TRANSLATION X FILE 'S.6GA51' FACTOR 1.0000
  END OF DYNAMIC LOAD
  LOAD ' TRANS'
  CQC LOAD ' Z DIR'
  LOAD 'LONGIT'
  CQC LOAD ' X DIR'
  LOAD COMB 'CASE 1' 'TRANS PLUS 0.3 TIMES LONGIT      CQC'
  LOAD COMB 'CASE 2' 'LONGIT PLUS 0.3 TIMES TRANS      CQC'
  PRINT STRUCTURAL DATA
  UNITS INCHES
  PLOT DEVICE PLOTTER PAPER 30
  PLOT FORMAT ORIENTATION NON STANDARD      24
  LABEL JOINTS

```

```

TITLE 'EXAMPLE PROBLEM'
TITLE BLOCK
JOB 'EXAMPLE PROBLEM'
FOR ' '
BY 'STRUBAG'
DISPLAY THREE DIMENSIONAL
PLOT FINISH
UNITS FEET
ASSEMBLE FOR DYNAMICS REDUCE BAND
INDEPENDENT DEGREE OF FREEDOM SELECT
CONDENSE DYNAMIC MATRICES
DUMP ORTHOGONALITY
MODAL ANALYSIS HOW 15                                $ <----- MODIFIED
LIST DYNAMIC NORMALIZED EIGENVECTORS
LIST DYNAMIC PARTICIPATION FACTORS
SHOCK SPECTRUM ANALYSIS LOAD ' Z DIR' ' X DIR'
GENERATE RESULTS FOR LOADS ' TRANS' 'LONGIT'
COMBINE 'CASE 1' ' TRANS' 1.0 'LONGIT' 0.3
COMBINE 'CASE 2' 'LONGIT' 1.0 ' TRANS' 0.3
LOAD LIST 'CASE 1' 'CASE 2'
$ LIST ELASTIC COL MEM FORCES (A*R*S LOAD)
LIST FORCES MEMBERS -
70211 70213 -
70311 70313 -
70321 70323
$ LIST ELAS. RESTRAINER FORCES (A*R*S LOAD)
LIST FORCES MEMBERS -
802
$ LIST ELAS. GLOBAL DISP FOR MAJOR JOINTS (A*R*S LOAD)
LIST DISPLACEMENTS JOINTS -
3011 1012 1021 2022 2023 1031 1035 3041
OUTPUT DECIMAL 4
$ LIST ELAS MEM FORCES AT ABUTS AND HINGES (A*R*S LOAD)
OUTPUT DECIMAL 1
LIST FORCES MEMBERS -
5011 5022 5035
$ |-----|
$ |          EXAMPLE 2          |
$ | V                          | V
OUTPUT DECIMAL 4                                $ <----- ADD
LIST FORCES MEMBERS -
71111 71113
$ LIST ELAS. GLOBAL DISP FOR MAJOR JOINTS (A*R*S LOAD)
LIST DISPLACEMENTS JOINTS -
3101 1102 1111 1115 3121
OUTPUT DECIMAL 4
$ LIST ELAS MEM FORCES AT ABUTS AND HINGES (A*R*S LOAD)
OUTPUT DECIMAL 1
LIST FORCES MEMBERS -
5101 5115
$ A                                A
$ |          EXAMPLE 2          |
$ |-----|
FINISH
CHANGES
JOINT 3041 REL MOMENT Y Z -
KFX 1.0 KFZ 1.0
ADDITIONS
PRINT JOINT RELEASES
$ BRIDGES WITH SHARP RADIUS CURVES(CURVES LESS THAN OR EQUAL
$ TO 1000.0 FEET) MAY GIVE A STATIC CHECK UNBALANCE MESSAGE.
$ IGNORE THIS MESSAGE AS THE RESULTS OF THE STATICS RUN ARE
$ SATISFACTORY.
$
INACTIVE MEMBERS -
802
LOADING 'W' 'WIND LOAD ON STRUCTURE'
MEMBER LOADS
5011 THRU 5015 FORCE Z GLOBAL PROJ UNI W 1.000

```


| | | | | | | | | | |
|-------|------|-------|-------|---|--------|------|-----|---|-------|
| 5021 | THRU | 5026 | FORCE | Z | GLOBAL | PROJ | UNI | W | 1.000 |
| 5031 | THRU | 5035 | FORCE | Z | GLOBAL | PROJ | UNI | W | 1.000 |
| 5011 | THRU | 5015 | FORCE | X | GLOBAL | PROJ | UNI | W | 0.240 |
| 5021 | THRU | 5026 | FORCE | X | GLOBAL | PROJ | UNI | W | 0.240 |
| 5031 | THRU | 5035 | FORCE | X | GLOBAL | PROJ | UNI | W | 0.240 |
| 70211 | THRU | 70213 | FORCE | Z | GLOBAL | PROJ | UNI | W | 1.000 |
| 70311 | THRU | 70313 | FORCE | Z | GLOBAL | PROJ | UNI | W | 1.000 |
| 70321 | THRU | 70323 | FORCE | Z | GLOBAL | PROJ | UNI | W | 1.000 |
| 70211 | THRU | 70213 | FORCE | X | GLOBAL | PROJ | UNI | W | 1.000 |
| 70311 | THRU | 70313 | FORCE | X | GLOBAL | PROJ | UNI | W | 1.000 |
| 70321 | THRU | 70323 | FORCE | X | GLOBAL | PROJ | UNI | W | 1.000 |

\$ |-----|
 \$ | EXAMPLE 2 |
 \$ V V

MEMBER LOADS

| | | | | | | | | | |
|-------|------|-------|-------|---|--------|------|-----|---|-------|
| 5101 | THRU | 5105 | FORCE | Z | GLOBAL | PROJ | UNI | W | 1.000 |
| 5111 | THRU | 5115 | FORCE | Z | GLOBAL | PROJ | UNI | W | 1.000 |
| 5101 | THRU | 5105 | FORCE | X | GLOBAL | PROJ | UNI | W | 0.240 |
| 5111 | THRU | 5115 | FORCE | X | GLOBAL | PROJ | UNI | W | 0.240 |
| 71111 | THRU | 71113 | FORCE | Z | GLOBAL | PROJ | UNI | W | 1.000 |
| 71111 | THRU | 71113 | FORCE | X | GLOBAL | PROJ | UNI | W | 1.000 |

\$ A A
 \$ | EXAMPLE 2 |
 \$ |-----|

LOADING 'WL' 'WIND LOAD ON LIVE LOAD'

MEMBER LOADS

| | | | | | | | | | |
|------|------|------|-------|---|--------|------|-----|---|-------|
| 5011 | THRU | 5015 | FORCE | Z | GLOBAL | PROJ | UNI | W | 0.100 |
| 5011 | THRU | 5015 | MOM | X | GLOBAL | PROJ | UNI | W | 0.850 |
| 5021 | THRU | 5026 | FORCE | Z | GLOBAL | PROJ | UNI | W | 0.100 |
| 5021 | THRU | 5026 | MOM | X | GLOBAL | PROJ | UNI | W | 0.850 |
| 5031 | THRU | 5035 | FORCE | Z | GLOBAL | PROJ | UNI | W | 0.100 |
| 5031 | THRU | 5035 | MOM | X | GLOBAL | PROJ | UNI | W | 0.850 |
| 5011 | THRU | 5015 | FORCE | X | | | UNI | W | 0.040 |
| 5011 | THRU | 5015 | MOM | Z | | | UNI | W | 0.340 |
| 5021 | THRU | 5026 | FORCE | X | | | UNI | W | 0.040 |
| 5021 | THRU | 5026 | MOM | Z | | | UNI | W | 0.340 |
| 5031 | THRU | 5035 | FORCE | X | | | UNI | W | 0.040 |
| 5031 | THRU | 5035 | MOM | Z | | | UNI | W | 0.340 |

\$ |-----|
 \$ | EXAMPLE 2 |
 \$ V V

MEMBER LOADS

| | | | | | | | | | |
|------|------|------|-------|---|--------|------|-----|---|-------|
| 5101 | THRU | 5105 | FORCE | Z | GLOBAL | PROJ | UNI | W | 1.000 |
| 5101 | THRU | 5105 | MOM | X | GLOBAL | PROJ | UNI | W | 8.750 |
| 5111 | THRU | 5115 | FORCE | Z | GLOBAL | PROJ | UNI | W | 1.000 |
| 5111 | THRU | 5115 | MOM | X | GLOBAL | PROJ | UNI | W | 8.750 |
| 5101 | THRU | 5105 | FORCE | X | | | UNI | W | 0.400 |
| 5101 | THRU | 5105 | MOM | Z | | | UNI | W | 3.500 |
| 5111 | THRU | 5115 | FORCE | X | | | UNI | W | 0.400 |
| 5111 | THRU | 5115 | MOM | Z | | | UNI | W | 3.500 |

\$ A A
 \$ | EXAMPLE 2 |
 \$ |-----|

LOADING 'LF' 'LONGITUDINAL FORCE'

MEMBER LOADS

| | | | | | | | | | |
|------|------|------|-------|---|--------|------|-----|---|-------|
| 5011 | THRU | 5015 | FORCE | X | GLOBAL | PROJ | UNI | W | 1.000 |
| 5011 | THRU | 5015 | MOM | Z | GLOBAL | PROJ | UNI | W | 8.500 |
| 5021 | THRU | 5026 | FORCE | X | GLOBAL | PROJ | UNI | W | 1.000 |
| 5021 | THRU | 5026 | MOM | Z | GLOBAL | PROJ | UNI | W | 8.500 |
| 5031 | THRU | 5035 | FORCE | X | GLOBAL | PROJ | UNI | W | 1.000 |
| 5031 | THRU | 5035 | MOM | Z | GLOBAL | PROJ | UNI | W | 8.500 |

\$ |-----|
 \$ | EXAMPLE 2 |
 \$ V V

MEMBER LOADS

| | | | | | | | | | |
|------|------|------|-------|---|--------|------|-----|---|-------|
| 5101 | THRU | 5105 | FORCE | X | GLOBAL | PROJ | UNI | W | 1.000 |
| 5101 | THRU | 5105 | MOM | Z | GLOBAL | PROJ | UNI | W | 8.750 |
| 5111 | THRU | 5115 | FORCE | X | GLOBAL | PROJ | UNI | W | 1.000 |

```

5111 THRU 5115 MOM Z GLOBAL PROJ UNI W 8.750
$ A
$ | EXAMPLE 2 |
$ |-----|
$
LOADING 'T' 'TEMPERATURE LOAD'
MEMBER TEMPERATURE LOADS
'/GP/SUPE' AXIAL 35.
'/GP/COLS' AXIAL 35.
'/GP/CAPS' AXIAL 35.
$
$ FOR BENT 2
$
$
LOADING 'DY - 2' 'TRANS MOMENT DISTRIBUTION'
$ FOR SINGLE COLUMN BENTS USE THE FOLLOWING COMMAND (ADD + 1 OR - 1 TO
$ THE ANSWER)
MEMBER 70213 END LOAD END MOMENT Y 1.0
$
LOADING 'DX - 2' 'LONGIT MOMENT DISTRIBUTION'
$ FOR SINGLE COLUMN BENTS USE THE FOLLOWING COMMAND (ADD + 1 OR - 1 TO
$ THE ANSWER)
MEMBER 70213 END LOAD END MOMENT Z 1.0
$
$ FOR BENT 3
$
$
LOADING 'DY - 3' 'TRANS MOMENT DISTRIBUTION'
MEMBER 6031 END LOAD START MOMENT Z 1.0
$
LOADING 'DX - 3' 'LONGIT MOMENT DISTRIBUTION'
MEMBER 6031 END LOAD START MOMENT X 1.0
$ |-----|
$ | EXAMPLE 2 |
$ V V
$
$ FOR BENT 11
$
$
LOADING 'DY - 11' 'TRANS MOMENT DISTRIBUTION'
$ FOR SINGLE COLUMN BENTS USE THE FOLLOWING COMMAND (ADD + 1 OR - 1 TO
$ THE ANSWER)
MEMBER 71113 END LOAD END MOMENT Y 1.0
$
LOADING 'DX - 11' 'LONGIT MOMENT DISTRIBUTION'
$ FOR SINGLE COLUMN BENTS USE THE FOLLOWING COMMAND (ADD + 1 OR - 1 TO
$ THE ANSWER)
MEMBER 71113 END LOAD END MOMENT Z 1.0
$ A A
$ | EXAMPLE 2 |
$ |-----|
$ STIFFNESS ANALYSIS REDUCE BAND
$ THE COMMAND ABOVE LISTS ALL RESULTS
OUTPUT DECIMAL 2
$ THE FOLLOWING COMMANDS REFORMAT THE COLUMN FORCES ONLY
OUTPUT BY MEMBER
$ LIST COLUMN MEMBER FORCES
LOADING LIST 'CASE 1' 'CASE 2' 'W' 'WL' 'LF' 'T' 'DY - 2', -
'DX - 2' 'CFH1- 2' 'CFH2- 2' 'CFH3- 2' 'CFP1- 2' 'CFP2- 2', -
'CFP3- 2'
LIST FORCES MEMBER -
70211 70213
LOADING LIST 'CASE 1' 'CASE 2' 'W' 'WL' 'LF' 'T' 'DY - 3', -
'DX - 3' 'CFH1- 3' 'CFH2- 3' 'CFH3- 3' 'CFP1- 3' 'CFP2- 3', -
'CFP3- 3'
LIST FORCES MEMBER -
70311 70313 -
70321 70323
$ |-----|

```

```
$ I          EXAMPLE 2          I
$ V          V
$ LIST COLUMN MEMBER FORCES
  LOADING LIST 'CASE 1' 'CASE 2' 'D' 'W' 'WL' 'LF' 'T' 'DY - 11', -
'DX - 11' 'CFH1- 11' 'CFH2- 11' 'CFH3- 11' 'CFP1- 11' 'CFP2- 11', -
'CFP3- 11'
  LIST FORCES MEMBER -
  71111 71113
  FINISH
```